ARCHITECT'S SUPPLEMENTAL INSTRUCTIONS

OWNER ARCHITECT CONTRACTOR **FIELD OTHER**

.EA - Architects, Architecture Planning Interiors 1730 East Northern Avenue, Suite 110 Phone: 602.943.7511 email: info@lea-architects.com

PROJECT: City of Mesa Public Safety Training Facility

(name, address) **Burn Facility Expansion**

3260 N. 40th Street

Mesa, AZ

OWNER: City of Mesa

20 East Main Street

Mesa, AZ 86336

JE Bowen Construction LLC TO: (Contractor)

517 S. Blossom

Mesa, AZ 85206

ARCHITECT'S SUPPLEMENTAL

INSTRUCTION NO: 1

DATE: March 7, 2016

MESA PROJECT NO: CP0096

ARCHITECT'S PROJECT NO: 140714

NTP DATED: Feb. 1, 2016

The Work shall be carried out in accordance with the following supplemental instructions issued in accordance with the Contract Documents without change in Contract Sum or Contract Time. Proceeding with the Work in accordance with these instructions indicates your acknowledgement that there will be no change in the Contract Sum or Contract Time.

Description: (Here insert a written description of the interpretation or change)

CITY OF MESA PUBLIC SAFETY TRAINING FACILITY **BURN FACILITY EXPANSION**

Sheet A0.0 – Cover Sheet	Sheet A7.3 – Garage Vents / Open Wall Joints
Sheet A3.0 – First Floor Plan	Sheet AA2.0 – Alternate Bid (Info Transferred to A3.0
Sheet A3.1 – Second Floor Plan	and A3.1)
Sheet A5.1 – Building Sections / Thermal Lining Details	Sheet S1 – Structural Notes and Details
Sheet A6.0 – Door Schedule / Door Details	Sheet S3 – Second Floor Framing Plan
Sheet A6.1 – Door Details	Sheet S4 – High & Low Roof Framing Plan
Sheet A6.2 - Door Details	Sheet S5 – Details
Sheet A6.3 - Door Details	Sheet S6 – Second Floor Details
Sheet A7.1 – Roof / Chopout Details	Sheet S8 – Stair Details

Revise as follows:

Sheet A0.0 - Cover Sheet

Cover Sheet revised to reflect ASI#1 and date

Sheet A3.0 – First Floor Plan / Sheet A3.1 – Second Floor Plan

Masonry Control Joint Locations (MCJ) as per GSN and Detail 2/A7.3 have been identified as per Revised Sheets A3.0 and A3.1

Note: Exact Locations within wall length may be adjusted based on masonry jointing/coursing within 8" of proposed locations.

- Door 201B has been relabeled 'Door Type A' on Sheet A3.1 as per attached revised Sheet A3.1
- Added General Floor Plan Note H. in reference to cutting of masonry units (8x8x16 / 8x8) in the middle of the wall - "Not at Top or Ends" as per Revised Sheets A3.0 and A3.1

Sheet AA2.0 - First Floor Plan / Second Floor Plan Alt. Bid

Floor Plan Layouts/ Room Finish Shcedule have been transferred to Sheets A3.0 and A3.1 since these Alternate Bids were accepted.

Sheet A5.1 – Building Sections / Thermal Lining Details

Det. 1 - Thermal Linings at Ceiling "Section with Thermal Lining Tile on Exterior" - Clarification that SS angle is not required were exterior Thermal Lining panels occur on the exterior as per Revised Sheets A5.1

PAGE 1

ARCHITECT'S SUPPLEMENTAL INSTRUCTIONS

OWNER ARCHITECT CONTRACTOR FIELD OTHER Architecture Planning Interiors
1730 East Northern Avenue, Suite 110
Phone: 602.943.7511
email: info@lea-architects.com

Construction Management Phoenix, Arizona 85020 Fax: 602.943.7784 www.lea-architects.com

PROJECT: City of Mesa Public Safety Training Facility

(name, address) Burn Facility Expansion

3260 N. 40th Street

Mesa, AZ

OWNER: City of Mesa

20 East Main Street

Mesa, AZ 86336

TO: (Contractor) JE Bowen Construction LLC

517 S. Blossom Mesa, AZ 85206 ARCHITECT'S SUPPLEMENTAL

INSTRUCTION NO: 1

DATE: March 7, 2016

MESA PROJECT NO: CP0096

ARCHITECT'S PROJECT NO: 140714

NTP DATED: Feb. 1, 2016

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Sheet A6.0 – Door Schedule / Door Details / Sheet A6.1 – Door Details / Sheet A6.2 - Door Details / Sheet A6.3 - Door Details

 Det. 7 – Steel Door/Window Head – Shows increased HTL lintel width to provide anchor point for Thermal Lining Tile (HTL) as per revised Sheet A6.0

Note: Increased HTL lintel width is to provide anchor point for Thermal Lining Tile (HTL) and is not due to structural loading.

 Door Types A, B, C and Window Shutters and associated steel hardware, supports ,etc. to be "Prime Painted Steel (Gray)" and NOT Hot-Dipped Galvanized due to the potential reaction with high heat as per attached revised Sheet(s) A6.0, A6.1, A6.2 and A6.3

Note: All other steel including, but not limited to stairs, Railings, scuppers, etc. shall remain Hot-Dipped Galvanized.

Sheet A7.1 - Roof / Chopout Details

 Det. 2 – Eave Detail @ Roof / Safety Railing System – Clarification showing in creased masonry top course (10") masonry top course to match Structural Det. 7/S7 as per revised Sheet A7.1

Sheet A7.3 - Garage Vents / Open Wall Joints

 Det. 2 – Plan Details-Open Wall Joints – Joint at Second Floor Columns modified to allow for expansion of masonry as per attached revised Sheet A7.3

Sheet S1 - Structural Notes and Details

• Masonry Note 18 - Revised Lintel Detail as per attached revised Sheet S1

Sheet S3 - Second Floor Framing Plan

- Line of Fire Brick shown at Stair as per attached revised Sheet S3
- Correct Det. 14/S6 keyed in as per attached revised Sheet S3
- Second Floor Framing Note 5 revised as per attached revised Sheet S3
- Note No. 6 removed as per attached revised Sheet S3

Sheet S4 – High & Low Roof Framing Plan

Roof Framing Note 5 revised as per attached revised Sheet S4

Sheet S5 - Details

 Det. 14 /S5 – Masonry & Concrete Lintels at Masonry Wall Openings – Deleted as per attached revised Sheet S5

Sheet S6 – Second Floor Details

 Det. 12 /S6 - Balcony Slab Bearing at Concrete Wall - Deleted CIP floor extension as per attached revised Sheet S6

ARCHITECT'S SUPPLEMENTAL INSTRUCTIONS

OWNER ARCHITECT CONTRACTOR FIELD OTHER Architecture Planning Interiors
1730 East Northern Avenue, Suite 110
Phone: 602.943.7511
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Construct
Phoenix,
www.le

Construction Management
Phoenix, Arizona 85020
Fax: 602.943.7784
www.lea-architects.com

PROJECT: City of Mesa Public Safety Training Facility

(name, address) Burn Facility Expansion

3260 N. 40th Street

Mesa, AZ

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20 East Main Street

Mesa, AZ 86336

TO: (Contractor) JE Bowen Construction LLC

517 S. Blossom Mesa, AZ 85206 ARCHITECT'S SUPPLEMENTAL

INSTRUCTION NO: 1

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Sheet S8 – Second Floor Details

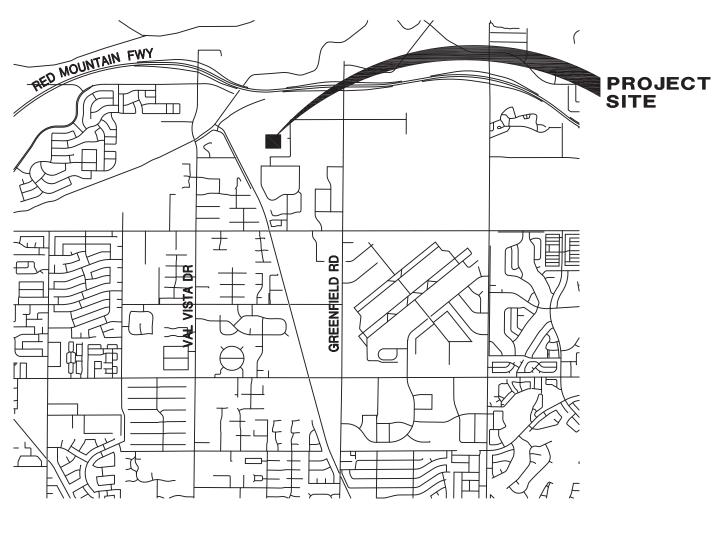
 Added Det. 7 /S8 – Typical Precast Concrete Lintels at Masonry Wall Openings - Revised deleting masonry lintel (steel Reinforcement) above the concrete lintel and clarifying that the precast lintel is "Refractory Concrete (HTL) to match Architectural Det. 7/A6.0 as per attached revised Sheet S8

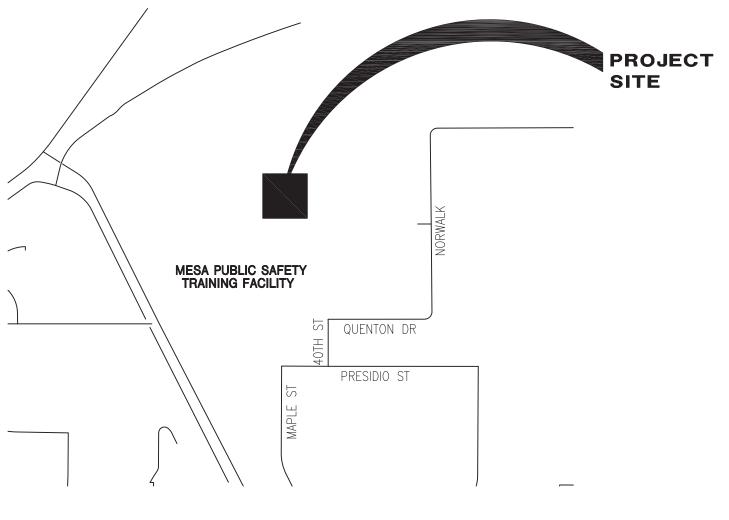
Attachments: (Here insert listing of attached documents that support description)

Revised Sheet A0.0 – Cover Sheet	Revised Sheet S1 – Structural Notes & Details
Revised Sheet A3.0 – First Floor Plan	Revised Sheet S3 – Second Floor Framing Plan
Revised Sheet A3.1 – Second Floor Plan	Revised Sheet S4 – High & Low Framing Plan
Revised Sheet A5.1 – Building Sections / Thermal Lining	Revised Sheet S5 – Foundation Details
Details	Revised Sheet S6 – Second Floor Details
Revised Sheet A6.0 – Door Schedule/Door Details	Revised Sheet S8 – Details
Revised Sheet A6.1 – Door Details	
Revised Sheet A6.2 – Door Details	
Revised Sheet A6.3 – Door Details	
Revised Sheet A7.1 – Roof/Chopout Details	
Revised Sheet A7.3 – Open Wall Joints	

ISSUED BY THE ARCHITECT:		
RJJ_ (signature)	Randy Jones RA, Senior Project Manager (Printed name and title)	







VICINITY MAP



MESA PUBLIC SAFETY TRAINING FACILITY BURN FACILITY EXPANSION

3260 N. 40TH STREET MESA, ARIZONA PROJECT NO. CP0096 PLAN SET 'A'

CITY OF MESA DEFERRED SUBMITTALS

1. FIRE PROTECTION CLASS 1 WET STANDPIPE

1. PER IBC SECTION 107.3.4.1, THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE SHALL BE RESPONSIBLE FOR REVIEWING AND COORDINATING SUBMITTAL DOCUMENTS PREPARED BY OTHERS, INCLUDING PHASED AND DEFERRED SUBMITTAL ITEMS, FOR COMPATIBILITY WITH THE DESIGN OF THE BUILDING.

LIVE FIRE TRAINING DESIGN CRITERIA

THE FIRE TRAINING PROP HAS BEEN DESIGNED FOR THE FOLLOWING CRITERIA:

- MAXIMUM NUMBER OF LIVE FIRE TRAINING DAYS PER YEAR = UNLIMITED

- MAXIMUM SUSTAINED TEMPERATURE DURING LIVE FIRE TRAINING IN BURN ROOMS = 1200 F
- MAXIMUM TEMPERATURE SPIKE DURING LIVE FIRE TRAINING IN BURN ROOMS = 1800 F
- ONLY "CLASS A" FUEL MATERIALS SHALL BE USED FOR LIVE FIRE TRAINING
- LIVE FIRE TRAINING SHALL OCCUR IN BURN ROOMS ONLY. NO FIRES ARE ALLOWED ON THE INTERIOR AND EXTERIOR STAIRS AND LANDINGS ON THE DECK OR ON THE ROOFS.
- LIVE FIRE TRAINING SHALL BE IN ACCORDANCE WITH NFPA 1403
- TRAINING THAT INCLUDES EXPLOSIVES, FIREARMS, OR TEAR GAS SHALL NOT BE PERMITTED WITHIN OR NEAR THE FIRE TRAINING PROP
- ONCE ALL CONCRETE AND MASONRY HAS BEEN COMPLETED, THE PROP SHALL STAND FOR A 2 MONTH MINIMUM CURING PERIOD BEFORE CONDUCTING THE FIRST FIRE LIVE TRAINING EVOLUTION. INSTALLATION OF OTHER TRADES MAY OCCUR DURING THE 2 MONTH CONCRETE AND MASONRY CURING PERIOD.
- 11. THE STRUCTURAL ELEMENTS HAVE BEEN PROTECTED FROM HEAT AND THERMAL SHOCK WITH THE THERMAL LININGS, NON-BEARING MASONRY WALLS AND OTHER NON-STRUCTURAL ITEMS ARE NOT PROTECTED WITH THERMAL LININGS, UNLESS OTHERWISE SHOWN ON THE DRAWINGS, AND ARE EXPECTED TO GRADUALLY DETERIORATE WITH EVERY EVOLUTION. MAINTENANCE WILL BE REQUIRED AND SHOULD BE INCLUDED IN ANNUAL BUDGETS.
- 12. FIRE SHOULD BE PLACED AWAY FROM DOORS, SHUTTERS, AND ROOF OPENINGS TO REDUCE DETERIORATION OF THOSE ITEMS

ELE A-ARCHITECTS, LLC

ARCHITECTURE PLANNING

PHOENIX, AZ

CONSTRUCTION MANAGEMENT INTERIORS PROJECT DATA & ZONING INFORMATION **DESIGN TEAM**

PHOENIX, AZ 85020

(602) 943-7511

MATERIAL LEGEND FINISH LUMBER ွဲ့တို့တို့ CRUSHED ROCK DIMENSION LUMBER GYPSUM BOARD C.I.P. CONCRETE GLASS (ELEVATION) GLASS (SECTION) //// batt insulation ---- water proofing SMALL SCALE METAL

SYMBOLS PROPERTY LINE EXISTING GRADE ELEV. **BUILDING SECTION** NEW GRADE ELEV. SECTION NUMBER SHEET NUMBER PARTIAL SECTION $\langle \rangle$ WINDOW TYPE DRAWING NUMBER DRAWING TITLE DRAWING SCALE X DETAIL NO. SHEET NO. DOOR NUMBER

APPLICABLE BUILDING CODES (WITH CITY OF MESA AMENDMENTS) 2006 INTERNATIONAL BUILDING CODE 2006 INTERNATIONAL MECHANICAL CODE 2006 INTERNATIONAL PLUMBING CODE 2006 INTERNATIONAL FUEL GAS CODE 2006 INTERNATIONAL FIRE CODE 2005 NATIONAL ELECTRIC CODE ICC/ANSI A117.1 & 2010 ADA STANDARDS

BUILDING OCCUPANCY

IS EQUAL TO, OR GREATER THAN 10 FEET

THIS STRUCTURE IS A FIRE TRAINING PROP-NOT AN OCCUPIED BUILDING. FIRE TRAINING EXERCISES WILL BE CONDUCTED USING CLASS 'A' FUELED BURNS. AT THE END OF EACH DAY THE FIRE DEPARTMENT WILL EXTINGUISH

PER TABLES 601 AND 602. EXTERIOR WALLS ARE NOT RATED, IF SEPARATION

BUILDING AREAS FIRST FLOOR: SECOND FLOOR W/ BALCONIES: 2,400 S.F. 4,725 S.F.

CONSTRUCTION TYPE: IIB **BASIC ALLOWABLE AREA (TABLE 503)** OCCUPANCY GROUP U: 8,500 S.F. 2 STORIES FIRE RESISTANCE FOR EXTERIOR WALLS PROJECT NAME MESA PUBLIC SAFETY TRAINING FACILITY BURN FACILITY EXPANSION PROJECT ADDRESS

PROJECT OWNER CITY OF MESA 20 E. MAIN STREET MESA, AZ 85211 (480) 644-3380

PROJECT USER

ARCHITECT ARCHITECTURAL LEA - ARCHITECTS, LLC LEA - ARCHITECTS, LLC 1730 E. NORTHERN AVE, SUITE 110

PROJECT DESCRIPTION THE MESA PUBLIC SAFETY TRAINING FACILITY

SR. PROJECT MANAGER: RANDY JONES, RA

PRINCIPAL: LAWRENCE ENYART, FAIA, LEED AP

NEW CONSTRUCTION "CLASS A" BURN PROP ON CAMPUS LOCATED AT 3260 N. 40TH STREET THE PROJECT INCLUDES VARIOUS TRAINING PROPS/ AREAS FOR PUBLIC SAFETY TRAINING

1730 EAST NORTHERN AVENUE SUITE 110 PHOENIX, ARIZONA 85020 CIVIL HUBBARD ENGINEERING, INC

MESA, ARIZONA 85210 STRUCTURAL **GERVASIO & ASSOC., INC.** PHOENIX, AZ 85012

ELECTRICAL

NP ENGINEERING PHOENIX, ARIZONA 85015

(602) 285-1720 (602)

(602)

(480)

943-7511

892-3313

265-1559



mesa·az

SHEET INDEX

1) A0.0 ARCHITECTURAL COVER SHEET
2) A1.0 OVERALL SITE PLAN/ EXIST. PHOTOS

ASI #01 2-24-16

ASI #01 2-24-16

A3.2 ROOF/ FLOOR PLAN

17) A7.1 ROOF DETAILS

A183402 23) AA2.0 ADDITIVE ALTERNATE BID

A183408 29) S6 SECOND FLOOR DETAILS
A183409 30) S7 HIGH/ LOW ROOF DETAILS
A183410 31) S8 STAIR DETAILS
A183411 32) S9 DETAILS

.183412 33) E1.0 ELECTRICAL SITE PLAN

A183416 37) C1.1 CIVIL COVER SHEET 4183417 - 38) C3.1 GRADING SHEET

A183418 39) C4.1 fireline Sheet

A183414 35) E3.0 ELECTRICAL FLOOR PLANS

A183413 34) E2.0 ELECTRICAL PHOTOMETRIC SITE PLAN

A183415 36) E4.0 ELECTRICAL SCHEDULES, DIAGRAMS, & CALCS

18) A7.2 ROOF/ MISC. DETAILS

20) A8.0 STAIR/ RAILING DETAILS

22) AA1.0 ADDITIVE ALTERNATE BID

21) A8.1 GATE/ HANDRAIL/ GUARDRAIL DETAILS

MESA PUBLIC SAFETY TRAINING FACILITY **BURN FACILITY EXPANSION**

PHOENIX, ARIZONA

ARCHITECTURAL COVER SHEET

260 N. 40TH STREET MESA, ARIZON

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CITY OF MESA

ENGINEERING DEPARTMENT

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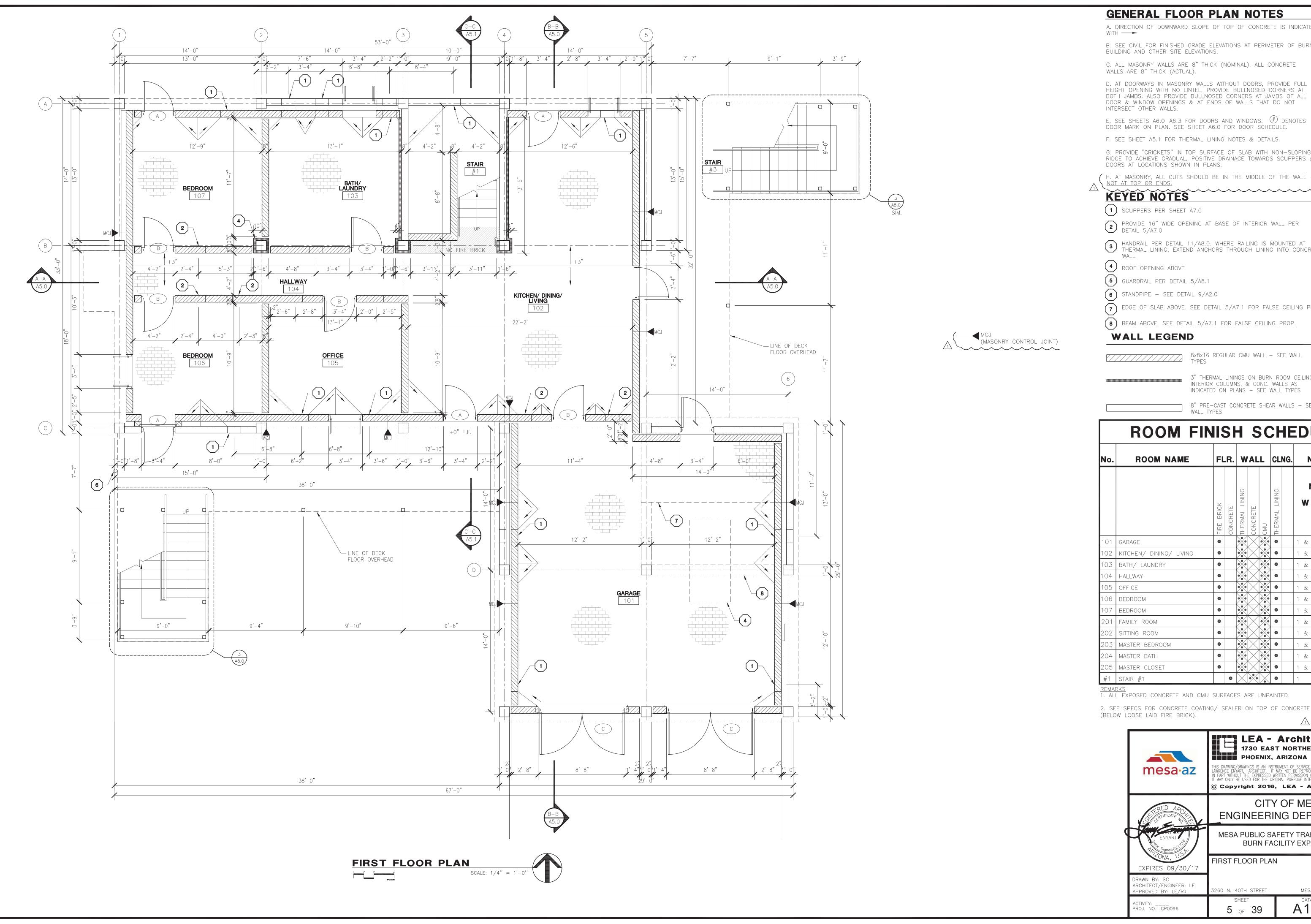
ARIZONA 81
Arizona Blue Stake, Inc.

PROJ. NO.: CP0096

RCHITECT/ENGINEER: LE

A183380

/₁\ ASI #01 2-24-16



A. DIRECTION OF DOWNWARD SLOPE OF TOP OF CONCRETE IS INDICATED

B. SEE CIVIL FOR FINISHED GRADE ELEVATIONS AT PERIMETER OF BURN BUILDING AND OTHER SITE ELEVATIONS.

C. ALL MASONRY WALLS ARE 8" THICK (NOMINAL). ALL CONCRETE

D. AT DOORWAYS IN MASONRY WALLS WITHOUT DOORS, PROVIDE FULL HEIGHT OPENING WITH NO LINTEL. PROVIDE BULLNOSED CORNERS AT BOTH JAMBS. ALSO PROVIDE BULLNOSED CORNERS AT JAMBS OF ALL DOOR & WINDOW OPENINGS & AT ENDS OF WALLS THAT DO NOT

F. SEE SHEET A5.1 FOR THERMAL LINING NOTES & DETAILS.

G. PROVIDE "CRICKETS" IN TOP SURFACE OF SLAB WITH NON-SLOPING RIDGE TO ACHIEVE GRADUAL, POSITIVE DRAINAGE TOWARDS SCUPPERS &

 $^\prime$ H. AT MASONRY, ALL CUTS SHOULD BE IN THE MIDDLE OF THE WALL -

3 HANDRAIL PER DETAIL 11/A8.0. WHERE RAILING IS MOUNTED AT THERMAL LINING, EXTEND ANCHORS THROUGH LINING INTO CONCRETE

T EDGE OF SLAB ABOVE. SEE DETAIL 5/A7.1 FOR FALSE CEILING PROP.

(8) BEAM ABOVE. SEE DETAIL 5/A7.1 FOR FALSE CEILING PROP.

8x8x16 REGULAR CMU WALL — SEE WALL TYPES

3" THERMAL LININGS ON BURN ROOM CEILINGS, INTERIOR COLUMNS, & CONC. WALLS AS

8" PRE-CAST CONCRETE SHEAR WALLS - SEE WALL TYPES

ROOM FINISH SCHEDULE

No.	ROOM NAME	FL	FLR.		WALL			CLNG.		NOTES		N
		FIRE BRICK	CONCRETE	THERMAL LINING	CONCRETE	CMU	THERMAL LINING		•	NORT W ••• S	1	
101	GARAGE	•			X		•		1	& 2		1
102	KITCHEN/ DINING/ LIVING	•			X		•		1	& 2		1
103	BATH/ LAUNDRY	•		 	$ \times $		•		1	& 2		1
104	HALLWAY	•			X		•		1	& 2		1
105	OFFICE	•			X		•		1	& 2		1
106	BEDROOM	•			X		•		1	& 2		1
107	BEDROOM	•			X		•		1	& 2		1
201	FAMILY ROOM	•			X		•		1	& 2		7
202	SITTING ROOM	•			X		•		1	& 2		2
203	MASTER BEDROOM	•			X		•		1	& 2		2
204	MASTER BATH	•			\times		•		1	& 2		2
205	MASTER CLOSET	•			X		•		1	& 2		2
#1	STAIR #1		•	X	%		0		1			

2. SEE SPECS FOR CONCRETE COATING/ SEALER ON TOP OF CONCRETE SLAB

LEA - Architects, LLC 1730 EAST NORTHERN AVE. NONE PHOENIX, ARIZONA

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ENGINEERING DEPARTMENT

CITY OF MESA

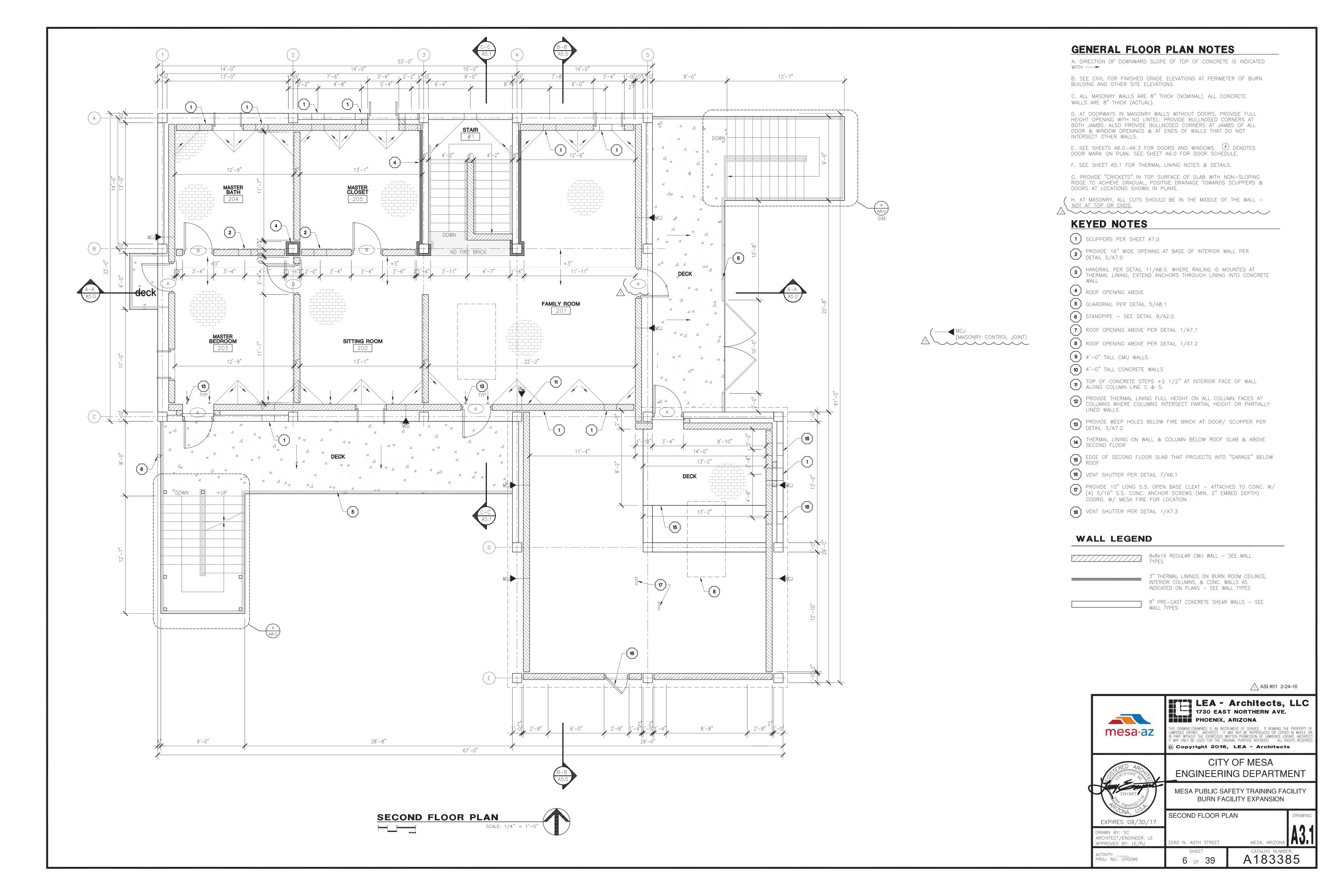
MESA PUBLIC SAFETY TRAINING FACILITY BURN FACILITY EXPANSION

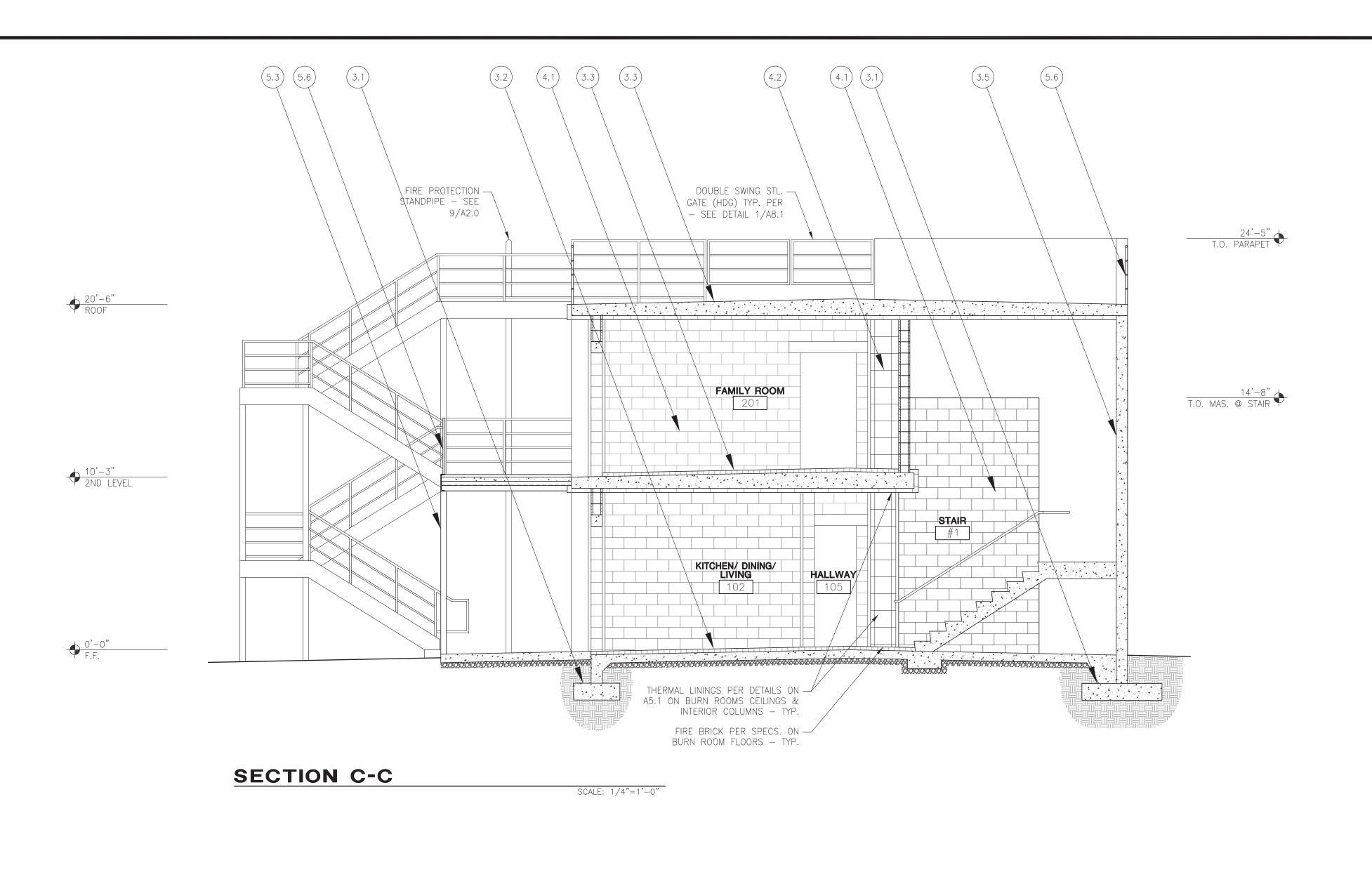
FIRST FLOOR PLAN

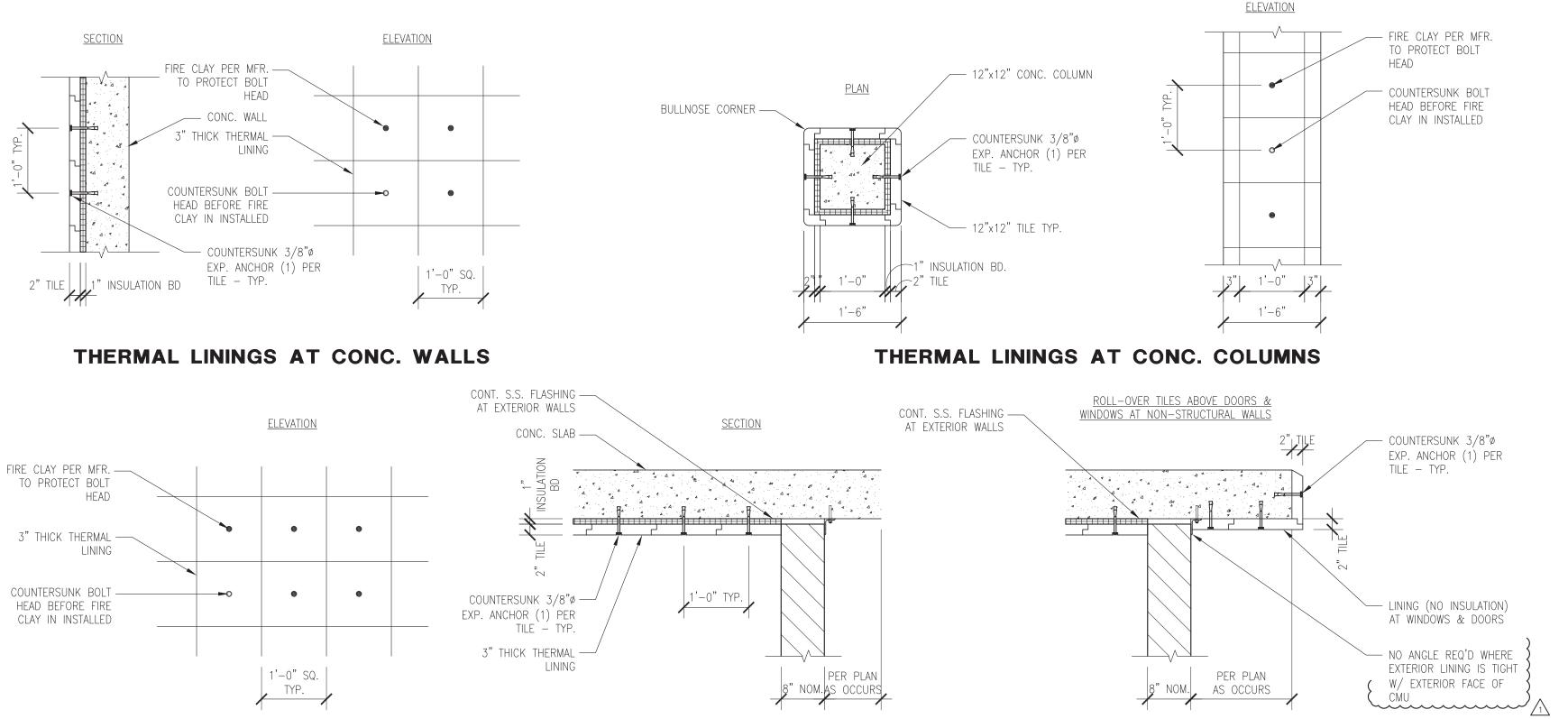
260 N. 40TH STREET

A183384

MESA, ARIZONA AVI







- 1. THE BASIS OF DESIGN FOR THE THERMAL LINING SYSTEM IS HTL SYSTEM 203 AND 203 LITE MANUFACTURED BY HIGH TEMPERATURE LININGS. SEE SPECIFICATION SECTION 07 00 00, THERMAL LINING SYSTEM FOR ADDITIONAL INFORMATION
- 2. INSTALL THERMAL LININGS IN ACCORDANCE WITH THE REQUIREMENTS OF THE MANUFACTURE. INSTALLATION SHALL BE PERFORMED BY A MANUFACTURE APPROVED INSTALLER
- 3. THE THERMAL LINING INSTALLER REQUIRES THE FOLLOWING SEQUENCE OF CONSTRUCTION TO COORDINATE
- INSTALLATION OF THERMAL LININGS AND MASONRY WALLS: A. CONSTRUCT THE CONCRETE FRAME AND STRUCTURE
- CONSTRUCT EXTERIOR MASONRY WALLS INSTALL THERMAL LININGS ON CEILINGS

THERMAL LINING

- CONSTRUCT INTERIOR MASONRY WALLS E. INSTALL BRACING ANGLES AT TOPS OF INTERIOR AND EXTERIOR WALLS
- 4. AS REQUIRED BY THE THERMAL LINING MANUFACTURER, THE OWNER WITH THE CONTRACTOR & THERMAL LINING (HTL) REPRESENTATIVE PRESENT, WILL PERFORM A "PRE—BURN" AT LEAST ONE—DAY BEFORE TRAINING BEGINS TO PROPERLY DRY OUT AND CURE THE THERMAL LININGS AS FOLLOWS:

 A. BURN 2 WOOD PALLETS AND A BAIL OF STRAW IN EACH ROOM THAT CONTAINS THERMAL LINING TILES
- B. ALLOW THE FIRE TO BURN UNTIL NEARLY EXHAUSTED
- AT THIS POINT, ADD 2 MORE PALLETS AND BURN AGAIN UNTIL NEARLY EXHAUSTED. REPEAT FOR A TOTAL OF 4 TIMES (8 PALLETSO
- LET THE FIRE BURN OUT COMPLETELY WITHOUT THE USE OF WATER TO EXTINGUISH THE FIRE. F. DO NOT BURN ALL 8 PALLETS AT THE SAME TIME
- 5. MANUFACTURE SHALL PROVIDE THE FOLLOWING OPERATIONS & MAINTENANCE INFORMATION: A. PER SPEC SECTION 01 78 23 O&M INFORMATION B. STANDARD OPERATING PROCEDURES
- PRE-BURN & CURING RECOMMENDATIONS D. BASIC MAINTENANCE

KEY NOTES

SEE DETAIL 5/A7.0 FOR CAST-IN-PLACE WALL JOINT DETAIL & STRUCT. (TYP. @ CAST-IN-PLACE CONC.)

- 3.1 CONC. FTG. SEE STRUCT
- 3.2 CONC. SLAB OVER BASE COURSE W/ FIRE BRICK AS OCCURS — SEE STRUCT. 3.3 CONC. SLAB W/ FIRE BRICK AS OCCURS
- 3.4 1/2" PRE-MOLD EXP. JOINT CAULK (TYP.)
- 3.5 CAST IN PLACE CONC. WALL W/ SNAP TIES 3.6 CAST CONC. PANEL - SEE STRUCT.
- 4.1 8x8x16 REGULAR CMU
- 4.2 THERMAL LININGS SEE 1/A5.1 FOR DETAILS

- 5.1 STL. BEAM SEE STRUCT.
- 5.2 STL. PLATE / HDG SEE STRUCT. 5.3 STL. TUBE / HDG - SEE STRUCT.
- 5.4 STL. WEB STIFFENER / HDG SEE STRUCT.
- 5.5 CHAIN LINK FALL PROTECTION SYSTEM 5.6 STL. GUARD RAIL OR HANDRAIL / HDG
- 5.7 BENT STL. PLATE / HDG SEE STRUCT.
- 5.8 STL. C CHANNEL / HDG SEE STRUCT. 5.9 STL. ANGLE / HDG — SEE STRUCT.
- 8.1 H.M. DOOR / HDG
- 8.2 H.M. FRAME / HDG
- 8.3 STL. WINDOW / HDG

8.4 STL. DOOR

/1 ASI #01 2-24-16



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EXPIRES 09/30/1

ARCHITECT/ENGINEER: LE

APPROVED BY: LE/RJ

DRAWN BY: SC

ACTIVITY: ____ PROJ. NO.: CP0096

CITY OF MESA ENGINEERING DEPARTMENT

MESA PUBLIC SAFETY TRAINING FACILITY BURN FACILITY EXPANSION

BUILDING SECTIONS/ THERMAL LINING DETAILS

MESA, ARIZONA I TY

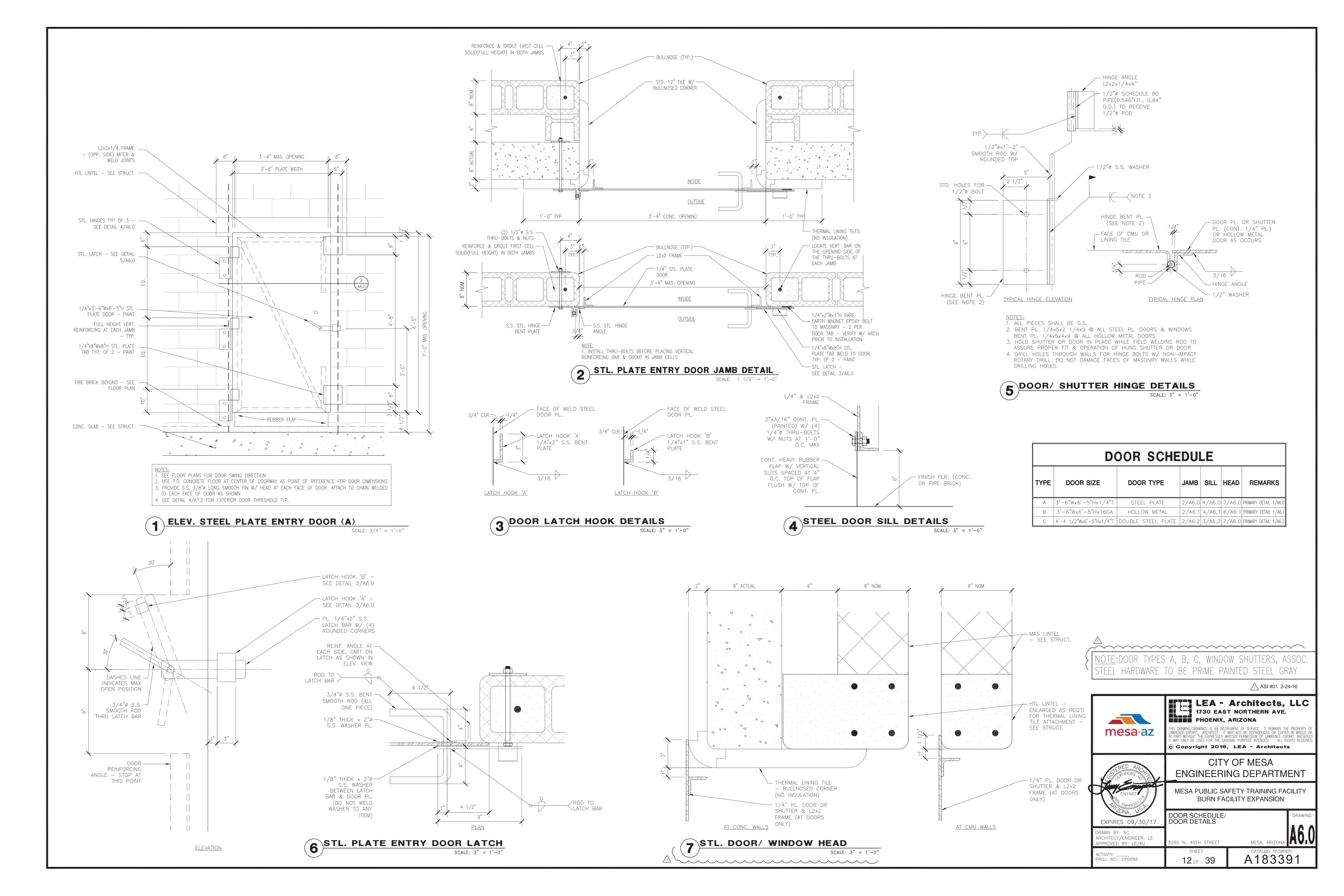
260 N. 40TH STREET

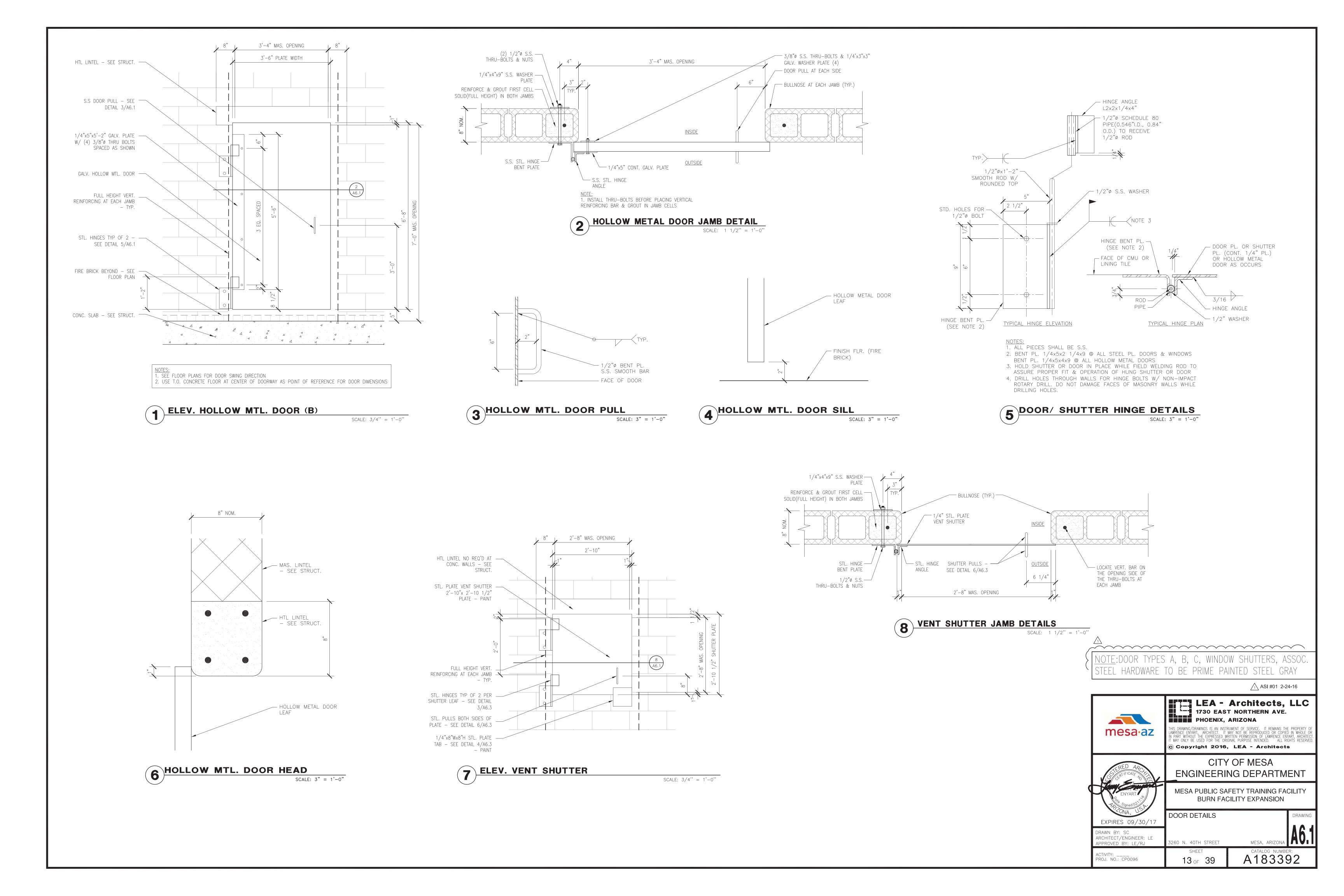
A183390 11 of 39

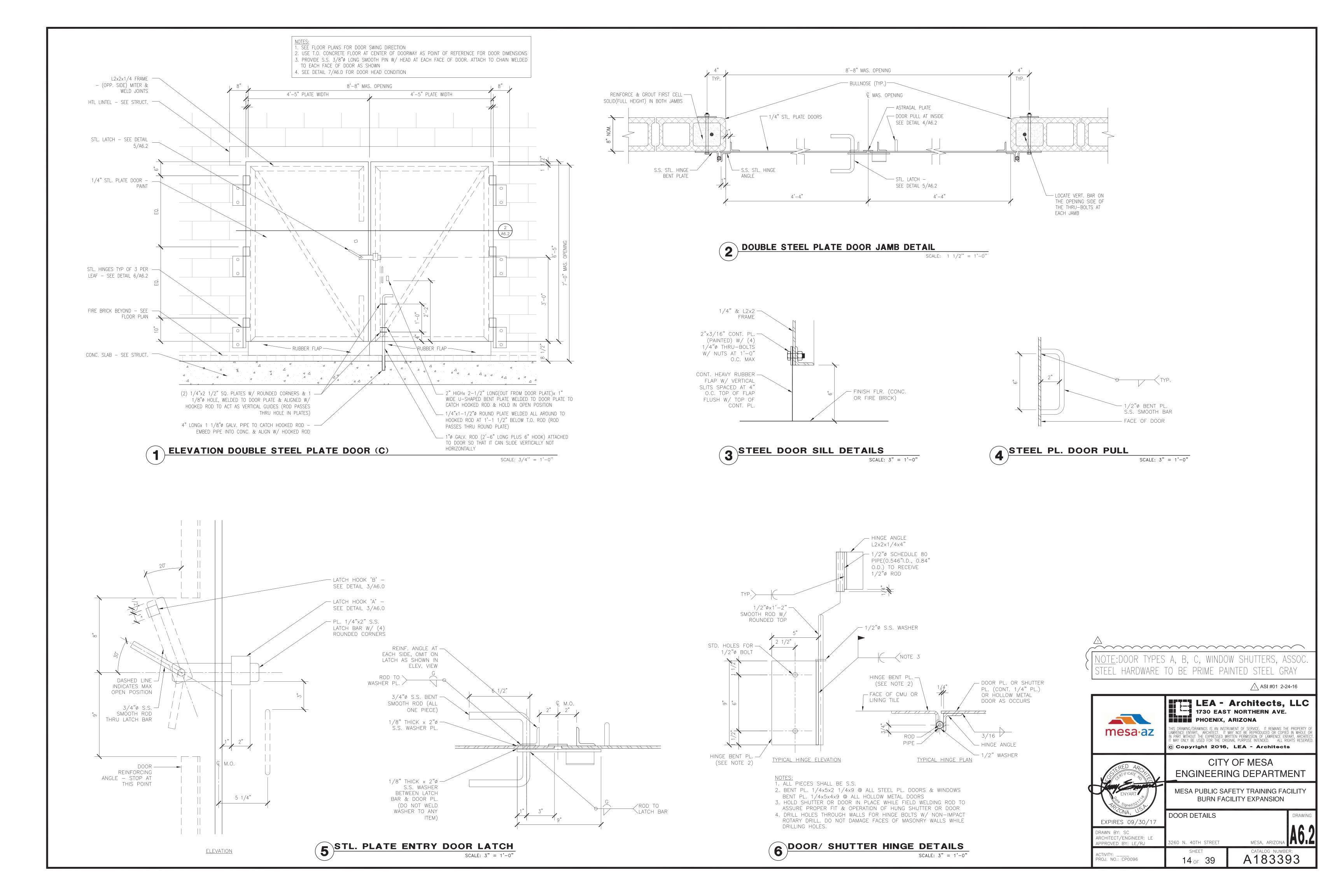
TYPICAL THERMAL LININGS DETAILS

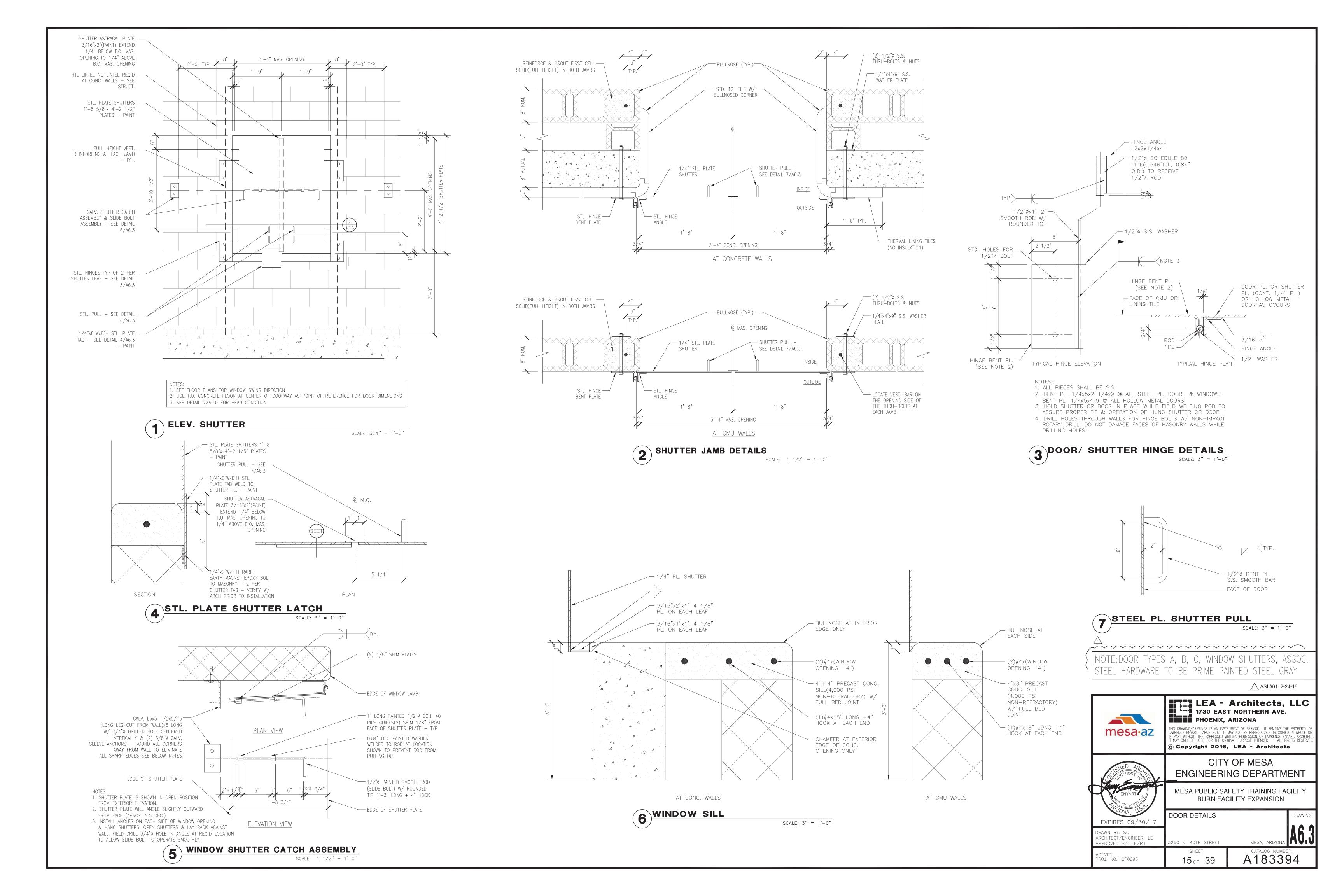
THERMAL LININGS AT CEILING

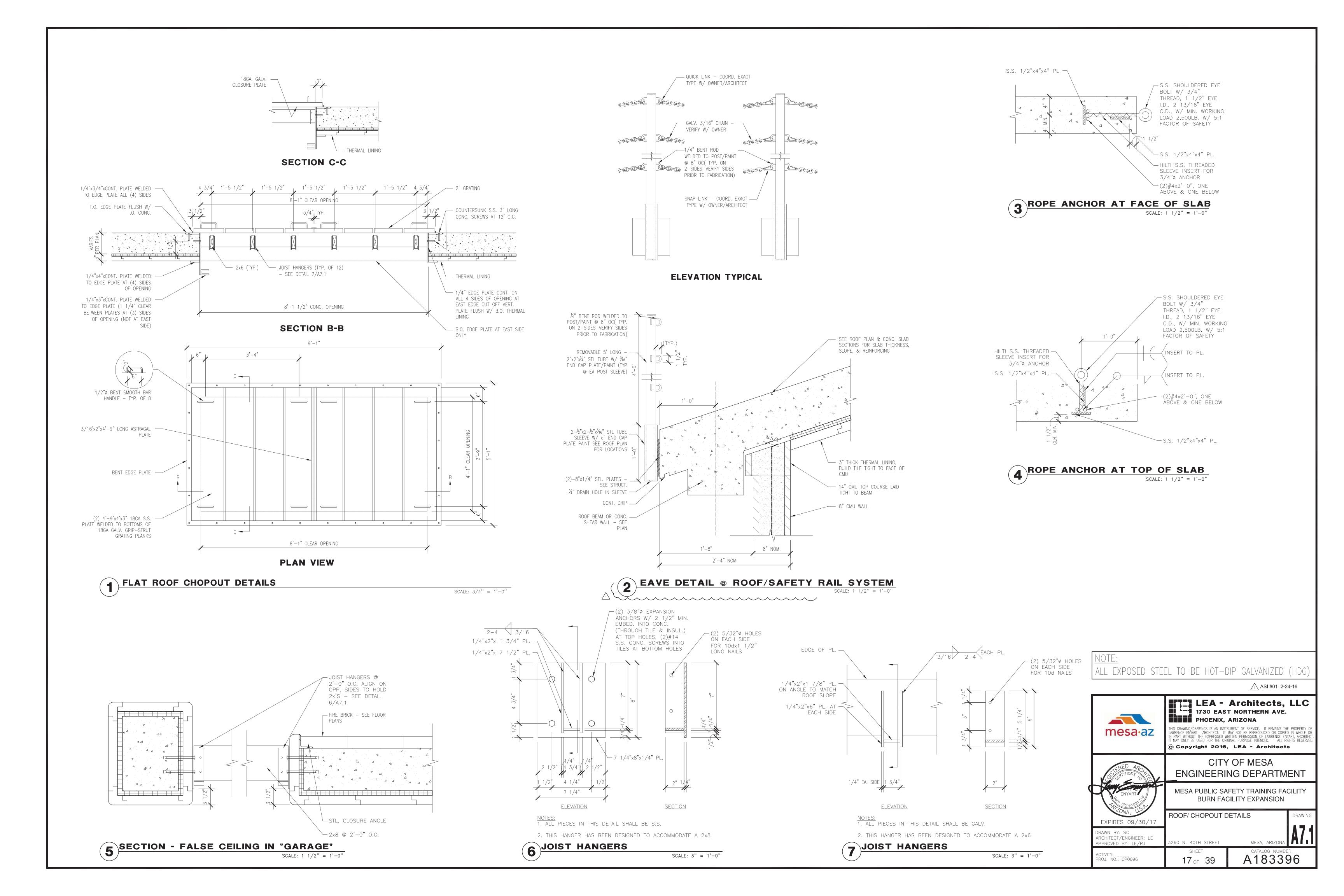
SCALE: 3/4'' = 1'-0''

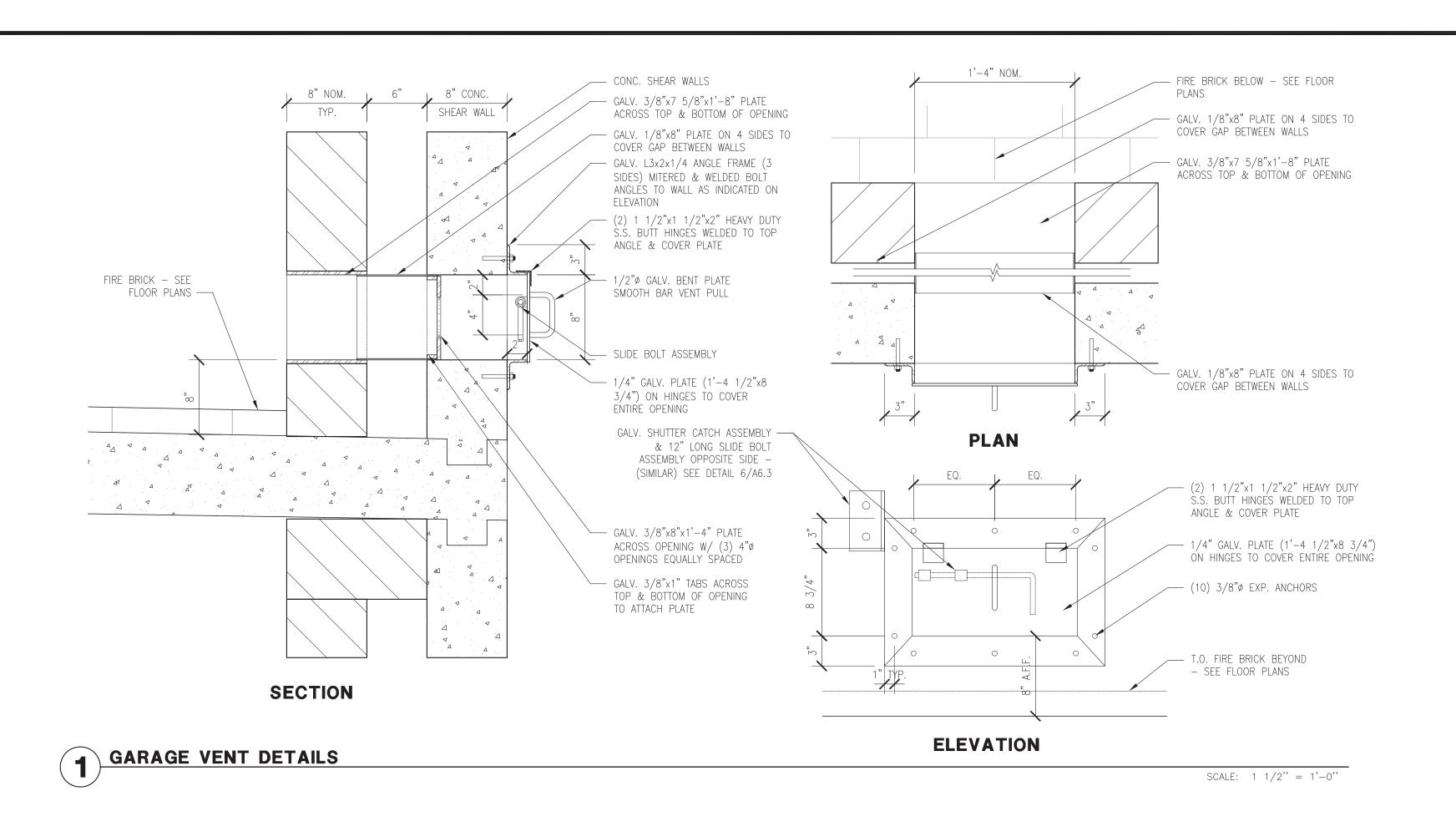


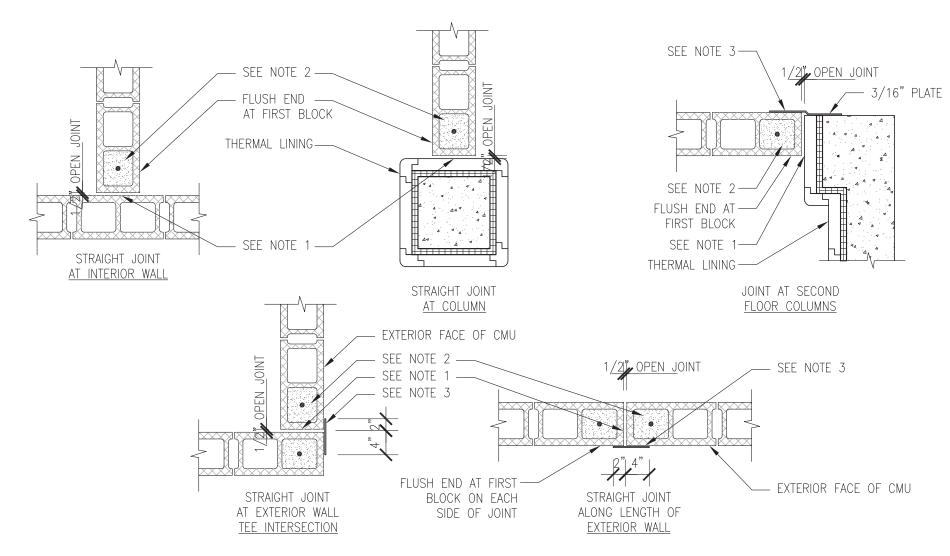






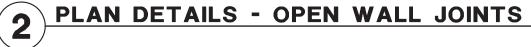






NOTES

- 1. FORM JOINT WITH 1/2" PLYWOOD OR COMPRESSIBLE STYROFOAM. STRIP PRIOR TO FIRST BURN.
- 2. REINFORCE & GROUT FIRST CELL SOLID FULL HEIGHT ON EACH SIDE OF JOINT.
- 3. AT EXTERIOR WALL JOINTS, PROVIDE S.S. 3/16"x6" PLATE TO COVER JOINT FOR FULL HEIGHT OF WALL, PROVIDE #14x1 3/4" LONG S.S. CRETE-FLEX SS4 MASONRY FASTENERS, #MF330, WITH HEX WASHER HEAD & ÖVERSIZED S.S. WASHER AT 2'-0" O.C. MAX. IN 5/16"x1 1/2" VERTICAL SLOTTED HOLES(MIN. 5 PER PLATE). FOR TWO TOP FASTENERS, PLACE FASTENER AT BOTTOM OF SLOT. FOR ALL OTHER FASTENERS, PLACE FASTENER IN THE MIDDLE OF SLOT. INSTALL FASTENERS UNTIL THEY ARE SNUG, BUT NOT OVER—TIGHTENED.



SCALE: 3/4'' = 1'-0''

ALL EXPOSED STEEL TO BE HOT—DIP GALVANIZED (HDG)



LEA - Architects, LLC
1730 EAST NORTHERN AVE.
PHOENIX, ARIZONA

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CITY OF MESA

ENGINEERING DEPARTMENT MESA PUBLIC SAFETY TRAINING FACILITY

BURN FACILITY EXPANSION

GARAGE VENTS/ OPEN WALL JOINTS

260 N. 40TH STREET

MESA, ARIZONA

ACTIVITY: ____ PROJ. NO.: CP0096

DRAWN BY: SC

EXPIRES 09/30/17

ARCHITECT/ENGINEER: LE

APPROVED BY: LE/RJ

A183398

CODE

1. Comply with 2006 International Building Code with City of Mesa Amendments.

DESIGN LOADS (General Case)

- Dead Loads:
 - a. High Roof: 119 psf (avg.) + 25 psf for thermal linings
 - Low Roof: 150 psf (avg.)+ 25 psf for thermal linings c. Second Floor: 119 psf (avg.) + 60 psf for CMU walls + 25 psf for
 - thermal linings + 27 psf for fire bricks. d. Mechanical Equipment & Piping: See plans
- Live Loads:
- a. High Roof: 50 psf
- b. Low Roof: 50 psf
- Second Floor: 110 psf (50 psf + 1 ft. water approx.)
- d. Stairs, Exit Corridors and Balconies: 100 psf
- Lateral Loads:
- c. Wind: 90 MPH, 3 second gust, Exposure C
- Risk Category II
- Internal Pressure Coefficient = ± 0.18
- d. Seismic: IE = 1.0, Risk Category II Ss = .211, SI = .067, Site Class C
- SDS = .169, SDI = .076, Seismic Design Category B
- Basic seismic-force-resisting system(s): Ordinary Reinforced Concrete Design base shear: 49.6k for 2 story area; 12.5k for one story area

FOUNDATIONS

- Soil report by Ricker, Atkinson, McBee, Morman & Associates, Inc. dated 4/8/2015, Project No. G22168.
- 2. Bear footings on firm, undisturbed site soils and/or compacted soil at 2.0 feet minimum below lowest adjacent finish grade. Finished grade is defined as lowest adjacent finished grade within 5'-0" of the perimeter of the building and top of floor slab for interior footings. See Detail 14/S1 for compaction and fill
- Place foundation concrete only on clean, firm, inspected bearing material. All footing excavations must be <u>inspected</u> by a qualified geotechnical engineer to insure proper foundation bearing.
- 4. Allowable soil bearing pressures:

Cs = .05, R = 4

- a. Footings on firm undisturbed or compacted soil: 2500 psf Refer to soils report for site grading, subgrade soil preparation, and fill and
- compaction requirements. 6. Foundations are designed for dry conditions and must remain dry during construction.

CONCRETE

- Concrete quality: Conform to ACI 301.
- 2. Concrete regular weight (144 pcf) with Type II cement per ASTM C150, aggregate per ASTM C33, and potable water. Replace cement with fly-ash at a rate of 15% to 20% by weight of the total cementitious materials. Fly-ash shall conform to ASTM C618, Type F.
- Aggregate size: 1" minimum (Size Nos. 5, 56 or 57) for footings, and other mass concrete and 3/4" minimum (Size No. 57) for other concrete.
- 4. Minimum 28-day compressive strength: Foundations: 3000 psi
- Slabs on Grade: 3000 psi; high range water reducers (superplastizers) required for slabs on grade.
- Walls: 4000 psi
- Columns: 4000 psi
- Suspended Slabs & Beams: 4000 psi
- Sidewalks, Curbs & Gutters: 2500 psi Not specified above: 4000 psi
- Maximum slump: 4-1/2" before high range water reducers (superplastizers) are
- added. Maximum water-cement ratio: 0.45. Mechanically vibrate all concrete, except that slabs-on-grade need be vibrated
- only around items embedded in the slab. Revibrate tops of columns. Form vibrators not allowed.

7. No admixtures without approval. Admixtures containing chlorides shall not be

- used. Concrete shall not be in contact with aluminum.
- 8. Do not cast walls or grade beams in lengths over 60'-0". 9. Wait 48 hours between adjacent concrete castings.
- 10. Cast slabs-on-grade with construction and control joints as shown on the plans.
- 11. Use roller tamp or vibrating screed for slabs. Hand tamping not allowed.
- 12. Cure concrete members with polyethylene for 5 days or with a curing and sealing compound per ASTM C1315 approved by the Architect. Apply 2 coats at right angles to each other.
- 13. Cold Weather Concreting: When the air temperature is below 50° for more than one half of any 24-hour period for three consecutive days, the requirements of ACI 306 (latest edition), Cold Weather Concreting practices shall apply. Concrete shall be protected from freezing until the concrete reaches 3500 PSI minimum compressive strength.
- 14. Hot Weather Concreting: Hot weather shall be any combination of high air temperature, low relative humidity and wind velocity resulting in evaporation rates which will impair the quality of fresh or hardened concrete. From March 1 through October 30 of each year (minimum), the requirements of ACI 305 (latest edition) Hot Weather Concreting shall apply. Limit the rate of evaporation to a maximum of 0.2 pounds per square foot per hour with the use of wind barriers, shade, foggers and concrete temperature. Evaporation retarder shall be used during the
- finishing process on all slabs. 15. Camber up all beams and slabs L/500 (in inches) at mid-span. Camber up all cantilevers L/300 (in inches) at end of cantilever.
- 16. Submit mix designs for review.
- 17. At the bottom of columns, before placing fresh concrete over hardened concrete, place a cement/sand slurry in the forms to a depth of 1-1/2". Slurry to be 1 part cement to 2 parts sand.
- 18. Do not place pipes, conduits, ducts, reglets, or chases in structural concrete without approval of the Structural Engineer through the Architect. See Architectural, Mechanical, and Electrical Drawings for locations.
- 19. Maximum free drop of any concrete 6'-0".
- 20. Non-shrink grout shall be non-metallic with a minimum compressive strength of 4000 psi in 3 days, mixed and installed in accordance with the manufacturer's recommendations. Non-shrink grout to be installed under beams and columns before loading members.

- 1. ACI Standard 347, latest edition, "Recommended Practice for Concrete
- Formwork" applies. Shoring below the slab may be removed and replaced with re-shoring when the
- concrete in the slab has attained a minimum strength of 3000 psi. Regardless of the concrete strength in the supported slab, allow a minimum of 7 days after the slab pour before removing the shores for re-shoring.

SHORING & RE-SHORING OF SUSPENDED CONCRETE SLAB

- All re-shoring operations shall follow <u>immediately</u> behind the removal of shores. Each re-shore shall be wedged tight and shall be load bearing.
- Maximum spacing of re-shores shall be 5'-0" o.c. until the supported concrete slab has reached design strength and a minimum of 28 days.
- 6. Excessive load concentrations from stacked material, equipment, etc. shall be
- 7. Contractor may provide additional shoring at his option.
- 8. Responsibility for adequacy of shoring rests with the Contractor.

REINFORCING

- 1. ASTM A615, Grade 60, except as follow:
- #2 and #3 bars ASTM A615: Grade 40. Beam and Girder Stirrups and Column Ties: ASTM A615, Grade 40.
- Welded Plain Wire Fabric: ASTM A185.
- Field bent and/or welded bars: ASTM A706.
- Reinforcing bars deformed except #2 bars and welded plain wire fabric. Concrete coverage for reinforcing bars (to face of bar including stirrups and ties) except as shown or noted:
- a. Unformed concrete in contact with earth: 3"
- Formed concrete in contact with earth: 2" Wall exterior face: 3"
- Wall interior face: 3"
- Suspended Slabs: 1 1/2" to bottom bars; 2" to top bars. Beams, girders and columns: 1-5/8" to ties; 2" to vertical bars.
- 4. Lap splices in concrete: 36 bar diameters u.n.o. on plans.
- a. Except as otherwise shown or noted, in beams and slabs not on grade, splice bottom bars over supports and top bars at mid-span only.
- b. Minimum 36 bar diameters. Stagger all splice locations 1 lap length
- minimum. See details for variation from minimum. c. Welded wire fabric splices: Mesh spacing + 2".
- 5. Lap splices in masonry: 45 bar diameters u.n.o. on plans.
- 6. Splice reinforcing only at approved locations. 7. Reinforcing spacings given are maximum on center and all reinforcing is
- continuous unless otherwise noted. 8. Provide bent corner reinforcing to match and lap with horizontal reinforcing at
- corners and intersections of walls, beams and footings per A.C.I. Detailing Manual
- Dowel all vertical reinforcing to foundations.
- 10. Securely tie all reinforcing and embedded items in position before placing concrete
- 11. Submit placing drawings per A.C.I. Detailing Manual (SP-66). Fabricate after Architect's review. Include elevations showing reinforcing steel at all concrete and masonry walls and footings.
- 12. Weld reinforcing bars in conformance with AWS D1.4-98. Use low hydrogen
- 13. Place reinforcing per ACI 318-05 and C.R.S.I. Standards.

MASONRY

- Hollow concrete block units: Grade N, Type 1, 1900 psi minimum compressive strength based on minimum net area. Wall design strength, f'm = 1500 psi. See "SPECIAL INSPECTION" section for masonry wall testing requirements.
- 2. Masonry construction requirements for hot and cold weather construction shall conform to "Specification for Masonry Structures" (ACI 530.1-05).
- 3. Lay units in running bond. Corners shall have a standard bond by overlapping
- 4. Mortar: Type S, 1800 psi minimum 28-day compressive strength.
- Grout: 2000 psi minimum 28-day compressive strength. Mechanically vibrate grout immediately after pouring. Puddling or rodding not allowed. Submit mix design for review. A maximum of 18% by weight of the total cementitious materials may be replaced by fly-ash, provided the fly-ash conforms to ASTM C618, Type F.
- Maximum grout lift without cleanouts 5'-0" in block walls. 7. Tie vertical reinforcing at each end and at 8'-0" maximum vertical spacing using single wire and loop type ties as manufactured by A.A. Wire Products Company or
- approved equal. 8. See Architectural Drawings for expansion and control joints. Locate at 24'-0" maximum but not less than 2'-0" from a bearing plate or jamb of an
- Grout stem walls solid. 10. In 8" walls provide vertical reinforcing in center of grout, at center of wall, continuous full height of wall as follows:
- a. For reinforcing at corners, intersections, wall ends, jambs and each side of expansion or control joints, see Detail 8/S5.
- b. 1 #4 at 32" o.c. typical vertical reinforcing. 13. Provide 1 - #4 in 8" minimum deep continuous grouted bond beams at all floor
- and roof lines and 1 #4 at top of all parapets. 14. See details for bond beams at floor and roof lines and other locations.
- 15. Stagger bond beam splices a minimum of 24".
- 16. Provide continuous wire lath or plastic mesh grout barriers below bond beams. 17. Provide ladder type #9 gage continuous joint reinforcing at 16" vertical spacing in
- 18. See Detail 7/S8 for lintels 1 19. Wet masonry walls thoroughly for three consecutive days immediately after placement. Omit wetting of masonry walls if temperatures will be below 38
- degrees during the next 24 hours. 20. All embedded anchor bolts, headed studs or strap anchors shall be grouted in place with a minimum of 4" of grout in the cell above and below the anchor with 1" grout all around bolt at face shell. See Detail 18/S5 for anchor embedments.
- 21. No expansion bolts will be allowed in masonry walls. GRATING
- 1. Grating shall be McNichols Co. GRIP STRUT, Heavy-Duty 1 1/2" deep x 12 gauge, 5-Diamond Plank (11 3/4" width).
- Grating shall be galvanized. 3. Floor grating shall be anchored to supporting members with galvanized steel saddle clips supplied by grating manufacturer bolted to fully welded galvanized headed studs.
- 4. Stair treads shall be McNichols Co. GRIP STRUT, 2" deep x 14 gauge, 5-Diamond Plank x 11 3/4" wide galvanized treads with standard nosing. Attach treads to channel stringers with 2 - 3/8" \phi galvanized thru-bolts at each end of tread.

STRUCTURAL STEEL, BOLTS & WELDS

- Latest AISC and AWS Codes and Handbooks apply.
- Rolled wide flange sections: ASTM A992, Grade 50, Fy = 50 ksi minimum. Plates, bars, angles and channels: ASTM A36, Fy = 36 ksi except for moment
- frame connections use ASTM A572, Grade 50.
- Pipes: ASTM A53, Grade B, Fy = 35 ksi minimum.
- Structural Tubes: ASTM A500, Grade B, Fy = 46 ksi. Bolts: ASTM A307, except where high strength bolts are specifically noted on
- Anchor bolts (straight, bent or headed): ASTM F1554 Standard Specification for
- High strength bolts: ASTM A325N. Use "snug-tightening" only.
- Welding rods: E-70 Series low hydrogen (i.e. only 7018 or 7028 electrodes). Do
- not use "Jet" welding rods (E7024) for any structural welding. 10. All stud anchors shall be fully welded, headed studs.
- 11. Accurately saw or finish column ends to a true plane.
- 12. At beam-to-beam or beam-to-column connections, where connection details are not provided, use AISC (ASD) Table II with a maximum number of 3/4" diameter A307 bolts for beam size shown, or equivalent Table III or IV connections.
- 13. Minimum connections to be 2 3/4" diameter A307 bolts or 3/16" fillet weld 4" long using 1/4" connection material detailed to minimize bending in connection.
- 14. All bolt holes through steel members shall be drilled or punched. Torch cut or "burned" holes are not allowed.
- 15. Comply with American Welding Society Codes and Standards. All welders shall hold valid certificates and have current experience in the type of weld called for. Certificates shall be those issued by an accepted testing agency authorized by
- 16. Steel surfaces to be welded shall be thoroughly cleaned of all foreign matter
- 17. Ultrasonically test <u>all</u> complete joint penetration (C.J.P.) welds. 18. Welds indicated with a shop weld symbol may be made in the field with approval of the Structural Engineer through the Architect.
- 19. Minimum embedment of bolts and anchors in masonry, grout or concrete. See

including paint for at least 2 inches from the root of the weld.

- 20. Submit shop drawings. Fabricate after Architect's review. 21. All structural shop welding shall be done in a city approved fabricator's shop, or be
- special inspected per IBC Section 1704.3. 22. All steel shall be hot dip galvanized.

SUPPLEMENTARY NOTES

- Verify all dimensions and conditions prior to starting work. Notify the Architect of
- any discrepancies or inconsistencies. Verify in field all existing conditions shown on drawings.
- Establish and verify all openings and inserts for mechanical, electrical and plumbing with appropriate trades, drawings.
- Provide all necessary temporary bracing, shoring, guying, or other means to avoid excessive stresses and to hold structural elements in place during construction. Options are for the contractor's convenience. He shall be responsible for all changes necessary if he chooses an option and shall coordinate all details. The cost of additional design work necessitated by selection of an option shall be
- borne by the contractor. Options shall not delay construction schedule. 6. The cost of additional design work due to errors or omissions in construction shall
- be borne by the contractor. Any engineering design provided by others and submitted for review shall bear the seal and signature of an Engineer registered in Arizona. If this engineering design requires special structural inspection, they shall be responsible for the inspection.
- Architectural Drawings. 9. Dimensions on the Structural Drawings are exact with the exception of masonry and sawn lumber dimensions which are nominal.

Details on the Structural Drawings are typical. Verify all dimensions with the

SPECIAL INSPECTION

Special inspection is to be provided in addition to the inspections conducted by the Department of Building Safety and shall not be construed to relieve the Owner or his authorized agent from requesting the periodic and called inspections required by Section 109 of the International Building Code. The special inspector shall be approved by the Cit Building Official prior to starting work.

SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING WORK:

CONCRETE:

(PER IBC SECTION 1704)

REINFORCING STEEL:

During the taking of test specimens and placing of reinforced concrete and pneumatically placed concrete.

During placing of reinforcing steel for all concrete required to have special inspection. This inspection may be done on a periodic basis in accordance with IBC Table 1704.4.

FIELD WELDING: During all structural field welding, including welding of reinforcing steel. This inspection

may be done on a periodic basis in accordance with IBC Section 1704.3.1.

EPOXY GROUTED ANCHORS:

During installation of epoxy grouted anchors.

DRYPACK & NON-SHRINK GROUT: During placement of drypack and non-shrink grout. This inspection may be done on a

periodic basis.

- **STRUCTURAL MASONRY: (Unit-strength tests)** 1. During sampling and placing of all masonry units, placement of reinforcement, inspection of grout space, immediately prior to closing of cleanouts and during all grouting operations.
- 2. Special inspection for the placing of the units shall be performed in accordance with IBC Section 1704.5 on a periodic basis. 3. The specified compressive strength of masonry, f'm, shall be verified by the "Unit
- Strength Method" as required by IBC Section 2105.2.2.1. a. The block units shall be tested for compressive strength prior to construction for each 5,000 square feet of wall area.

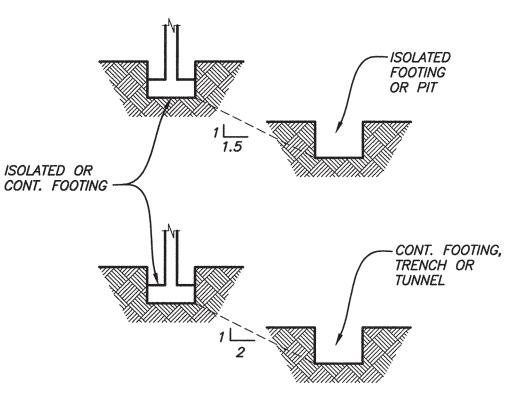
b. Separate grout and mortar testing is required for each 5,000 square feet of

wall area. Mortar shall be tested in accordance with ASTM C780. Grout shall

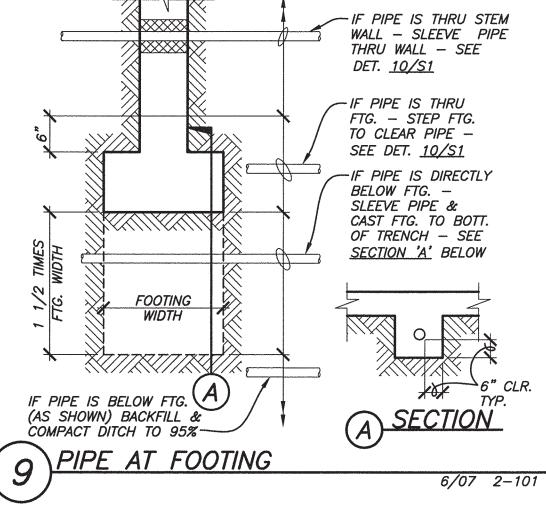
SITE GRADING, EXCAVATION AND FILLING:

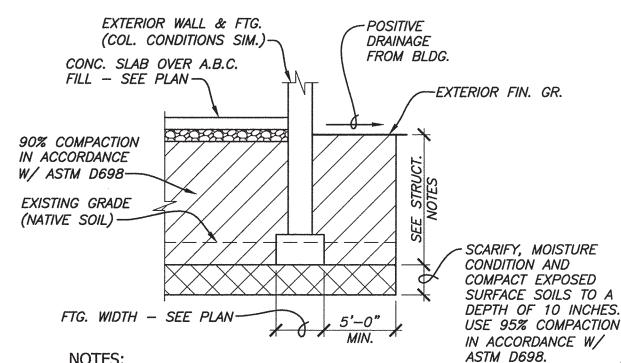
During earthwork excavations, grading and filling operations inspection to satisfy requirements of Chapter 18 and Section 1704.7 of the International Building Code. A qualified geotechnical engineer shall be responsible for this special inspection.

be tested in accordance with ASTM C1019.



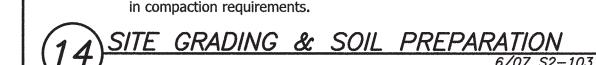
SEPARATION OF EXCAVATION

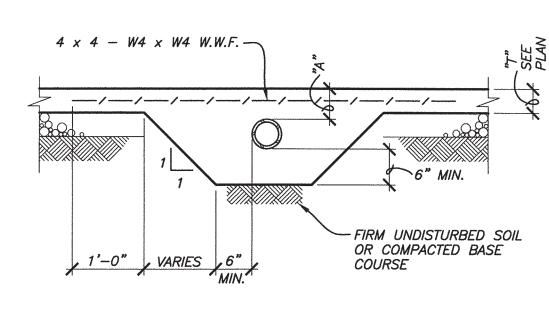




NOTES:

- 1. Site grading and subgrade soil preparation shall conform with
- recommendations as outlined in soils report. All site work shall be inspected by a qualified geotechnical engineer. If cemented soils are encountered, contact soils engineer for modification

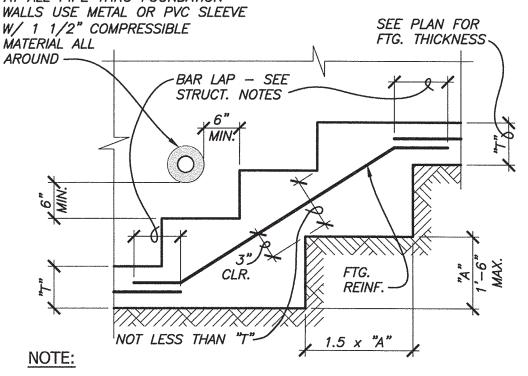




1. THIS DETAIL APPLIES ONLY IF PIPE IS SPECIFICALLY REQ'D. IN SLAB. DO NOT PLACE PIPE IN SLAB UNLESS REQ'D. OR APPROVED BY STRUCTURAL ENGINEER.

2. DIMENSION "A" TO BE EQUAL TO "T" OR 4" MIN. WHICHEVER IS GREATER.

PIPE IN SLAB ON GRADE AT ALL PIPE THRU FOUNDATION

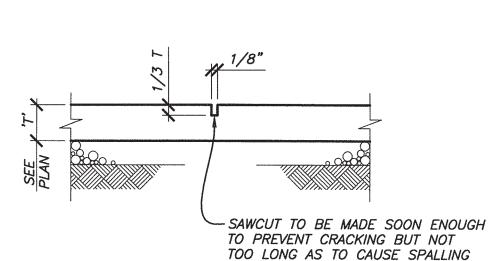


STEP IN WALL FOOTING

Use this detail to:

1. Change footing elevation.

2. Lower footing for firm bearing.



SAWCUT JOINTS IN SLAB ON GRADE

1 \ ASI #01 2-24-16

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LEA - Architects, LLC 1730 EAST NORTHERN AVE.

ENGINEERING DEPARTMENT MESA PUBLIC SAFETY TRAINING FACILITY **BURN FACILITY EXPANSION**

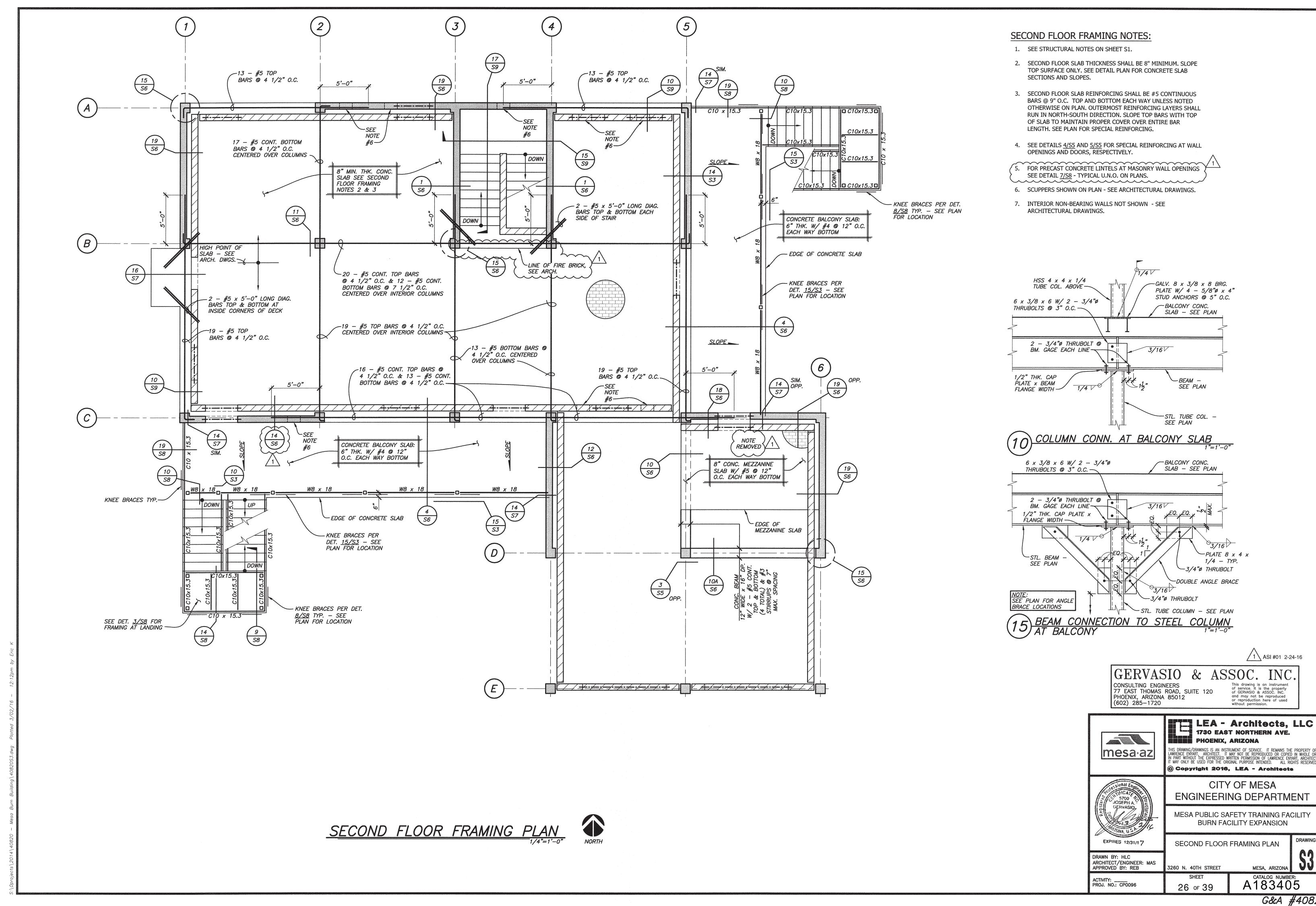
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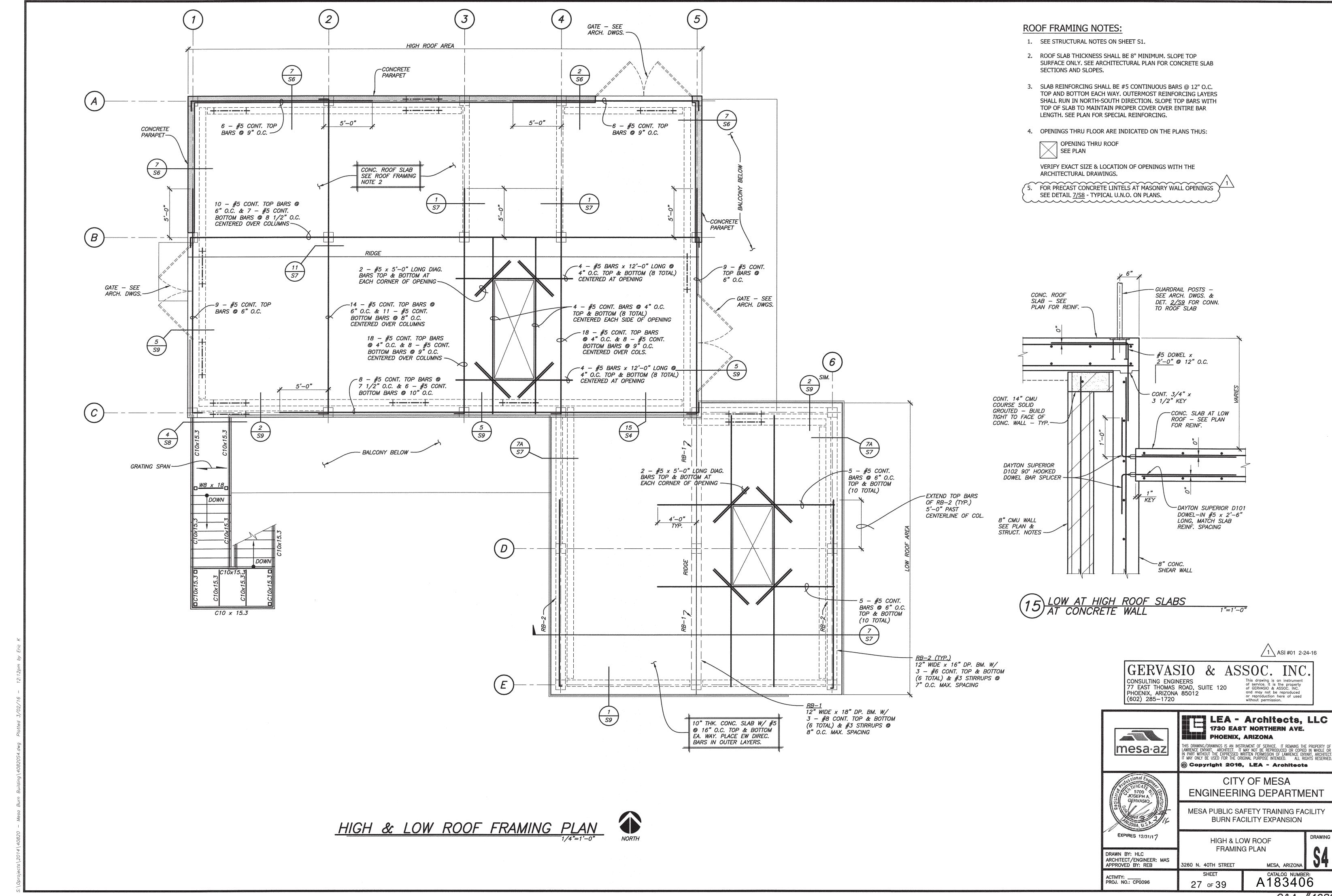
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3260 N. 40TH STREET

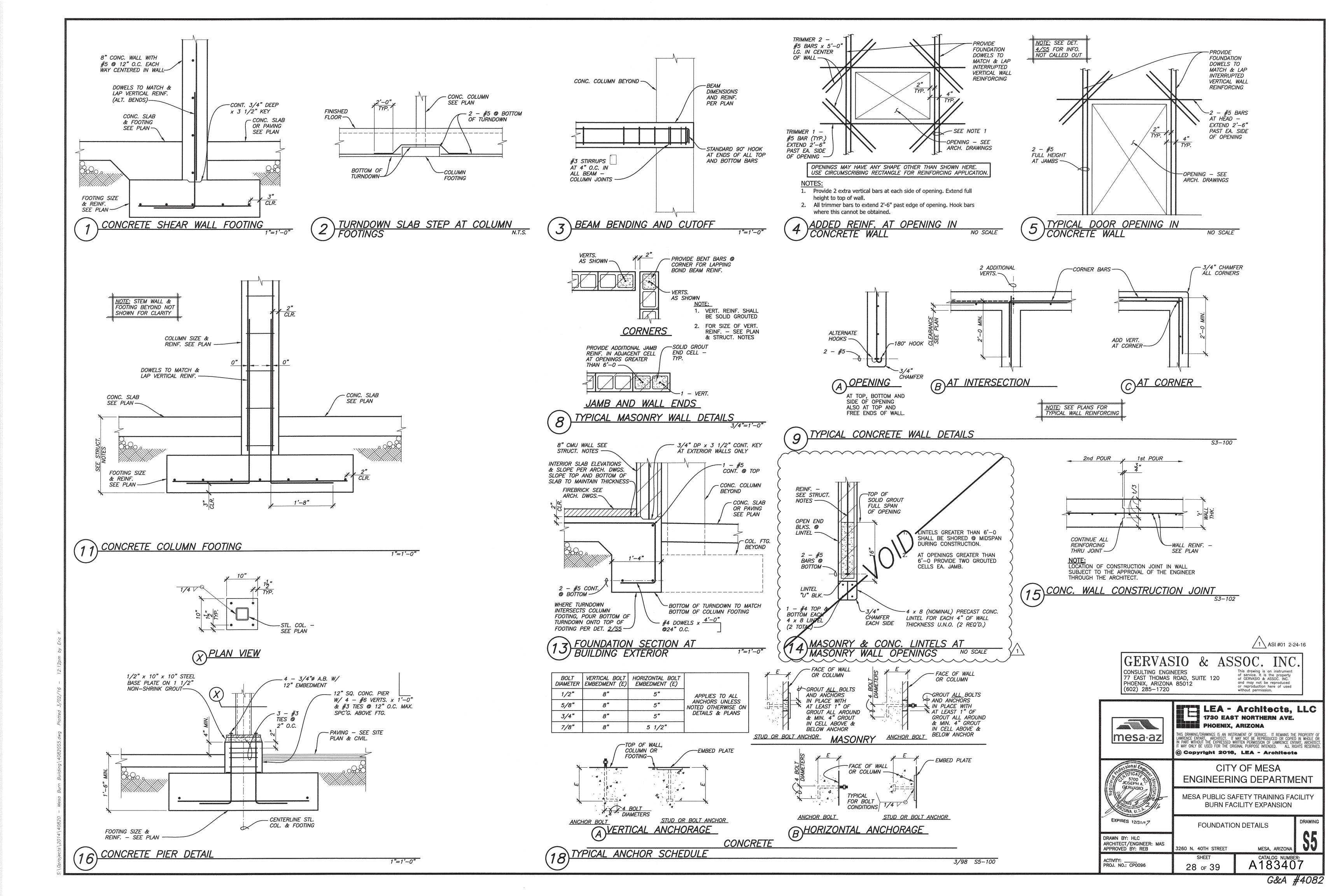
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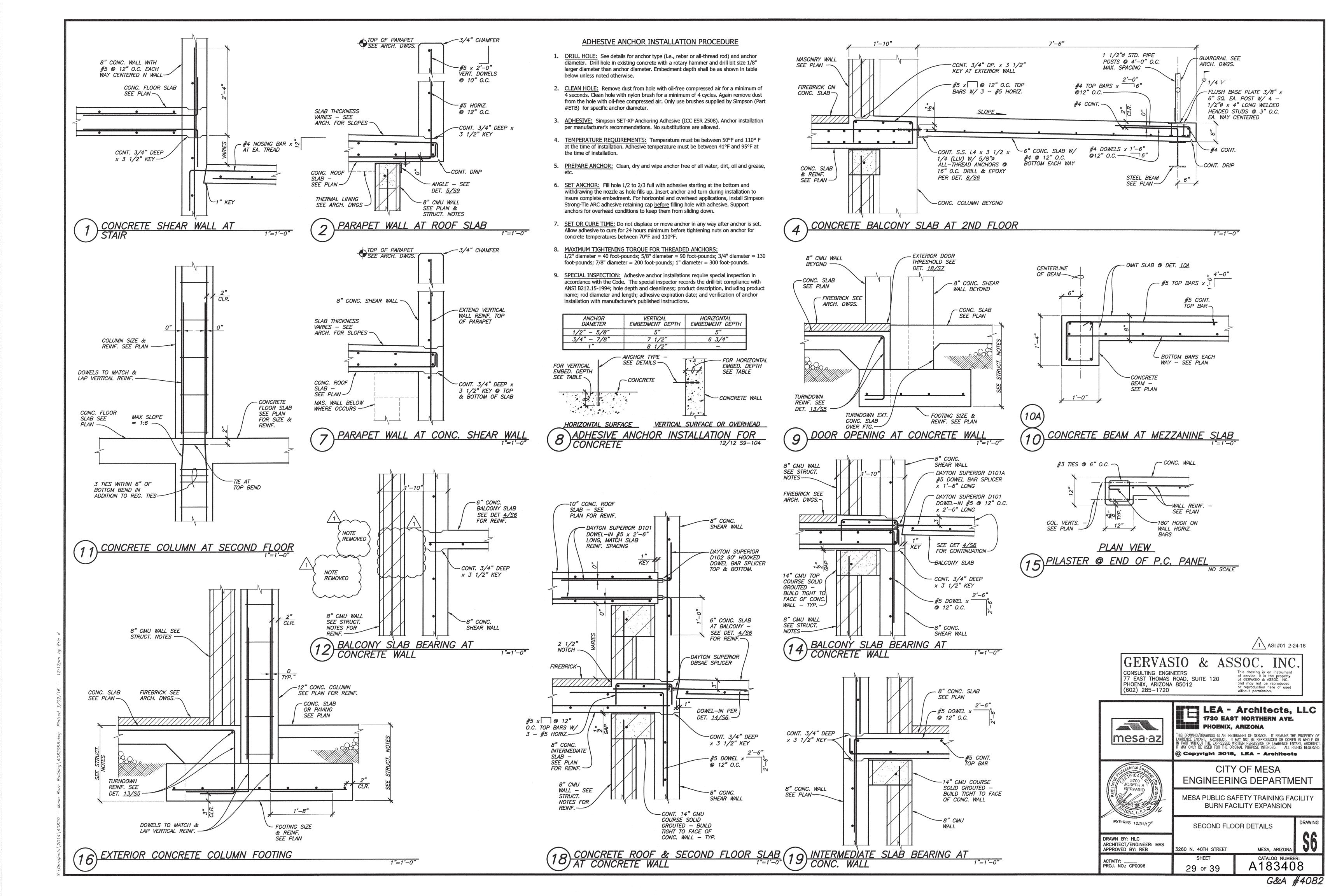
MESA, ARIZONA

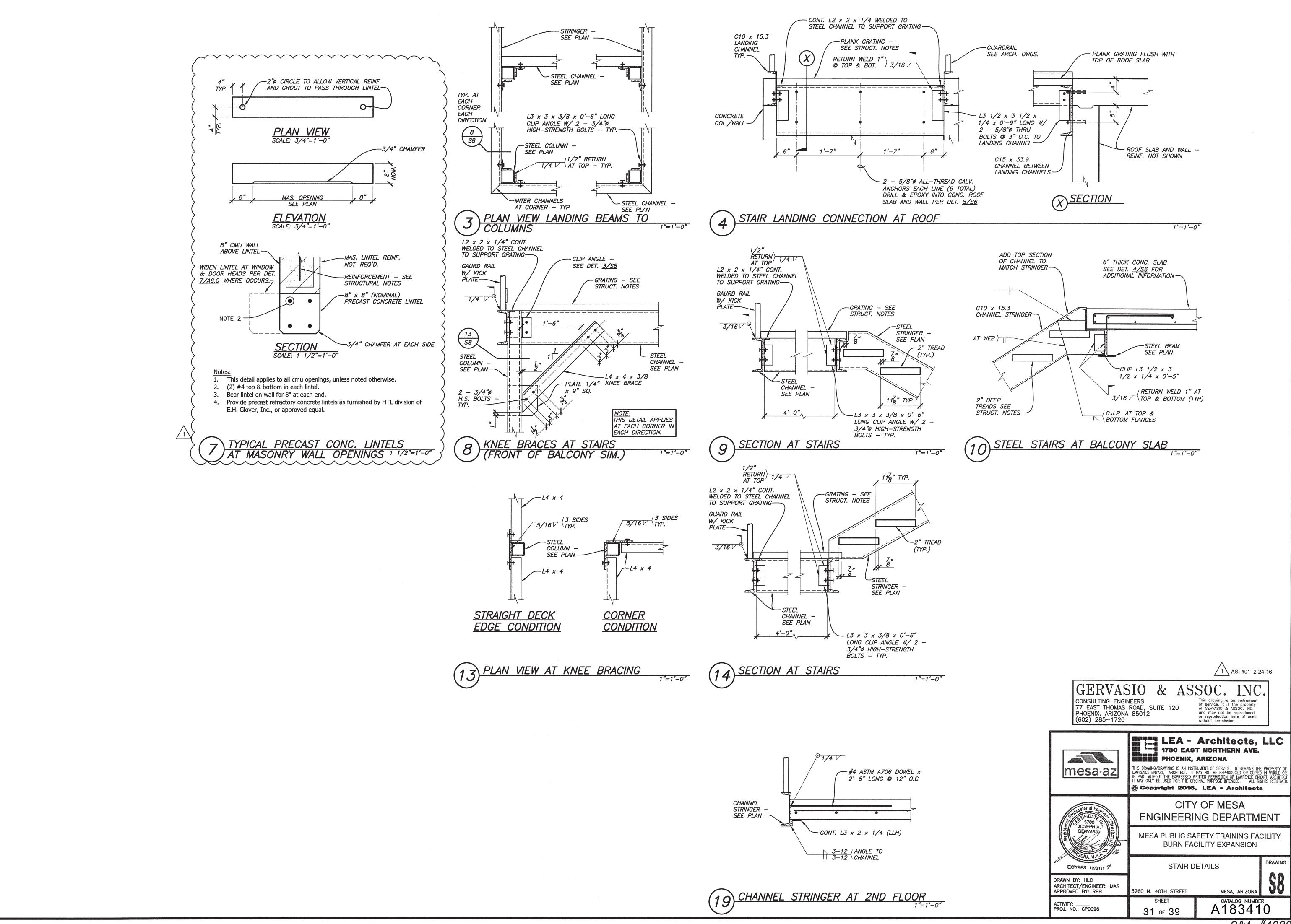




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